

Final Report

## Metroplan Orlando Travel Time Study \& Benefit-Cost Analysis

"Travel Time Studies and Benefit-Cost Analysis for Signal Retiming Projects covering Orange, Seminole, and Osceola Counties in the Central Florida Region"

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Metroplan Orlando
\&
GMB Engineers \& Planners, Inc.

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## 1 Introduction

### 1.1 Overview

Metroplan Orlando has requested GMB Engineers \& Planners, Inc. (GMB) to assess the benefits of the recently completed signal retiming projects on twenty-four (24) selected roadways spread throughout the tri-county (Orange, Seminole, and Osceola) area in the Central Florida region. Out of the 24 study roadways, nine fall within Seminole County, seven (7) fall with Orange County, six (6) fall within the City of Orlando, and the remaining two (2) fall within Osceola County. The study roadways for each of these four (4) jurisdictions are depicted in Figures 1 through 4 in the following pages. A list of the study roadways with information on segment limits, length, and maintaining jurisdiction is provided in Table 1.

To determine whether the benefits from the completed signal retiming projects would outweigh the implementation costs, a Benefit-Cost analysis was performed for each of the study roadways using the input parameters collected during the Travel Time and Delay (TTD) studies conducted before (before scenario) and after (after scenario) the implementation of retiming plans. The TTD studies were conducted for all the 24 twenty-four (24) study roadways in the before scenario. However, based on the direction from the City of Orlando project staff, TTD studies for the after scenario were conducted on only 21 study roadways. Due to the ongoing construction and other reasons, the signal retiming on Hoffner Avenue/Narcoossee Road, Mills Avenue, and Central Boulevard in the City of Orlando was postponed until the next year.

This report, in particular, presents the results of the TTD studies and the Benefit-Cost analysis for these recently completed signal-retiming projects in the study area.

### 1.2 Background

Signal re-timing projects generally demonstrate positive results with measurable benefits such as reduced delay, fuel savings, improved air quality, and others. Signal re-timing is one of the most cost-effective strategies to improve traffic flow, enhance safety, and lessen driver frustration. As part of the periodical signal retiming projects to improve the traffic flow on selected study roadways in Central Florida (Study Area), Florida Department of Transportation (FDOT) has recently completed signal re-timing on those roadways for the year 2010. GMB's role is to conduct TTD studies to assess the benefits achieved through these signal-retiming projects.





Table 1: List of Study Roadways

| Roadway <br> Name | Segment <br> Limits | Length <br> (Miles) | Jurisdiction |
| :---: | :---: | :---: | :---: |
| Alafaya TI (SR 434) | Challenger Pkwy Intersection | NA | Orange |
| Aloma Ave (SR 426) | Phelps Ave to Palmetto Ave | 2.00 | Orange |
| Colonial Dr E. (SR 50) | Murdock Blvd to Avalon Park Blvd | 4.10 | Orange |
| Conway Rd (SR 15) | Michigan Ave to Hoffner Ave | 2.30 | Orange |
| Goldenrod Rd (SR 551) | Bates Rd to Charlin Pkwy | 6.80 | Orange |
| Orange Ave (SR 527) | Drennan Rd to Nela Ave | 2.70 | Orange |
| Semoran Blvd (SR 436) | Aloma Ave to Baldwin Park St | 2.20 | Orange |
| Curry Ford Rd (SR 552) | Conway Rd to Woodgate Blvd | 2.20 | Orlando |
| Hoffner Ave/ <br> Narcoossee Rd (SR 15) | Goldenrod Rd to Lee Vista Blvd | 1.20 | Orlando/Orange |
| Semoran Blvd (SR 436) | Dahlia Dr to T.G. Lee Blvd | 6.00 | Orlando |
| Colonial Dr (SR 50) | Mills Ave to Old Cheney Hwy | 2.65 | Orlando |
| Mills Ave (SR 15/600) | Marks St to Lake Shore Dr/Rollins St | 1.20 | Orlando |
| Central Blvd | Brown Ave to Mills Ave | 0.15 | Orlando |
| US 17/92- US 441 | Osceola Pkwy to Columbia Ave | 2.00 | Osceola /Kissimmee |
| US 192 - US 441 | Denn John Ln to Turnpike NB Ramp | 4.00 | Osceola |
| US 17/92 | SR 46 (1st St) to 3rd St | 0.13 | Seminole |
| SR 434 | Sand Lake Rd to Jamestown Blvd | 0.20 | Seminole |
| SR 434 | Tollgate Trail to Wayman St | 2.54 | Seminole |
| SR 434 | Mitchell Hammock Rd to Palm Valley Dr | 2.77 | Seminole |
| SR 436 | Wilshire Dr to Casselton Dr | 3.10 | Seminole |
| SR 46 | Park Dr to Sanford Ave | 0.19 | Seminole |
| CR 46A | SR 417 NB Ramp to SR 417 SB Ramp | 0.12 | Seminole |
| SR 426 | Old Howell Branch Rd to Dean Rd | 2.30 | Seminole |
| Red Bug Lake Rd | SR 417 EB to SR 417 WB | 0.20 | Seminole |

## 2 Travel Time \& Delay Studies

### 2.1 Overview

For the TTD studies, a unique, safe, and innovative technology was used, which utilizes the integration of GPS and Geographical Information Systems (GIS) based technologies for data collection and reduction purposes. The GPS approach has proven to be cost-effective, safer, and more accurate than other methods. The before and the after travel time data on the study roadways were collected using the GeoStats In-Vehicle GeoLogger GPS equipment and floating car technique. GIS and GPS based software tool (TRAVTIME) was used to reduce the field collected travel time data. The output from the before and after TTD studies: 1) travel time data and 2) fuel consumption were utilized in calculating the benefit cost ratios for the study roadways.

### 2.2 Background

According to the Manual on Uniform Traffic Studies (MUTS), TTD studies are conducted to evaluate the quality of traffic movement along a route, by time of day and direction and determine the locations, types, and extents of traffic delays experienced at predefined locations or points by using a moving test vehicle. The data collected in the field are used to compute various Measures of Effectiveness (MOEs) for determining the quality of traffic movement. Some of the important MOEs calculated from the field data collection include average travel time, average travel speed, average delay time, and fuel consumption.

Travel time is a direct measure of the performance of the roadway network. High travel times are an indication of congestion, delay, loss of time by drivers, increased fuel use and increased pollution emissions. The travel time data collected can be an important component of the Congestion Management Process (CMP), which alerts the decision makers of progress toward meeting congestion and mobility goals, when collected on a regular basis.

### 2.3 Methodology

### 2.3.1 Study Prerequisites

For conducting a Travel Time and Delay Study, the following study prerequisites are generally fulfilled.

Study Area: The study roadways defined for this project are illustrated in Figures 1 through 4 and Table 1.

Control Points: For the purposes of this study, all the signalized intersections were considered as the control points for each study roadway. The information on signalized intersections was collected from the respective counties and FDOT Roadway Characteristics Inventory (RCI) Database.

Number of Study Runs: A procedure to determine the number of study runs in each direction is specified in Chapter 14 of the MUTS. However, for the purposes of this study, the Metroplan Orlando project staff specified that a minimum of four (4) study runs should be completed for each study route in each direction.

Data Collection Schedule: A data collection schedule is developed, taking into account scheduled roadway construction and school vacation periods, which would affect the results.

### 2.3.2 Study Procedure

GMB committed four (4) vehicles equipped with GeoStats In-Vehicle GeoLogger to this project. This ensured that the data collection could be completed within the project schedule and allowed time for any roadway segments that may be affected by severe weather or other factors. The before travel time data for the study roadways were collected between second week of January 2010 and second week of February 2010. The after field travel time data were collected anywhere between second week of March 2010 and first week of June 2010, depending on the completion of the signal retiming project for an individual study roadway.

The field data were collected from Tuesday through Thursday during the morning and afternoon peak periods. For each peak period and direction of travel, a minimum of four (4) vehicle runs will be completed for all study roadways. Based on previous experience of collecting Travel Time and Delay data for Metroplan Orlando and Seminole County, GMB realized that the congestion might not extend on the study roadways through the entire two hours between 7:00 and 9:00 a.m. for the morning and between 4:00 and 6:00 p.m. for the afternoon.

To correct this situation and to capture the actual peak travel of each road segment, GMB, as an innovative solution to obtain accurate data used the most current traffic count data from Orange, Seminole and Osceola Counties and from FDOT to determine the actual peak hour (between 7 to 9 a.m. and 4 to 6 p.m.) of travel. The data were collected with run start and end times within the actual peak hour. Within the time-period selected, GMB technicians utilized the entire peak hour for collecting the data.

By following this procedure the technicians collected a minimum of four (4) vehicle runs and in the majority of the cases collected additional runs (more than four).

In performing the data collection, a control point was established at least 1000 feet upstream of the first signal or at the first available median opening of each direction/route. All the signals within the roadway segments were considered as control points. The roadway segments were divided based on the control points identified in the signal-retiming project.

The technicians took field notes describing any factors or conditions that may affect the traffic operations. As a rule, data collection runs were not performed when external factors such as inclement weather, traffic incidents, special events, or roadway construction affected the typical traffic flow of the study roadway. The weekly schedules provided to the field technicians helped them to pursue the backup routes in case of accidents, special events or other factors that may affect the validity of the data.

The data collected for each roadway segment for each period and direction included street name, beginning and ending cross street, jurisdiction, facility type, area type, number of through lanes, left turn and right turn lanes, length, average travel time, stop delay, traffic control device, average travel speed, and speed limit. The procedures described above that were used in collecting the data for the "before" conditions prior to the signal timing plans are implemented were followed in the case of "after" conditions after the signal timing plans are implemented also.

### 2.3.3 Data Analysis

The GPS data collected were used to determine directly the following four (4) crucial parameters for each of the study roadways during the identified peak hour before and after a retiming plan has been implemented. The four (4) travel parameters are defined as follows:

Average Travel Time: The average time needed to travel between two control points.
Average Travel Speed: The average speed of travel between two control points, including all delays. It is calculated by dividing the total length of the section under consideration by the Average Travel Time.

Average vehicle Delay Time: The average delay time experienced between two points due to any kind of obstruction to the free flow speed that would otherwise occur during ideal traffic conditions (in the absence of traffic control, in the absence of geometric delay, in the absence of any incidents, and when there are no other vehicles on the road).

Fuel Consumption: The amount of fuel consumed during the travel between two control points.
Out of these four (4) parameters, Average Total Travel Time and Fuel Consumption were the main input parameters for assessing the effectiveness of the completed signal retiming process.

## 2,3.4 Level of Service Calculation

Level of Service (LOS) is one of the vital measures used to evaluate intersection or roadway performance. LOS was calculated before and after a retiming plan is implemented. Using the Average Travel Speed and roadway class (predetermined using the posted speed limit) as inputs, the roadway, or intersection LOS was determined using the HCM (2000) Exhibit 15-2 Urban Street LOS by Roadway Class and Average Travel Speed. The HCM (2000) Exhibit 15-2 is shown as Table 2.

Tables showing the TTD study results for each study roadway are provided in Appendix A of this report. In addition, GIS (Geographical Information System) maps graphically illustrating the LOS conditions and listing the travel time and delay summaries are also provided in Appendix A of this report.

Table 2: HCM Exhibit 15-2 - Urban Street LOS by Roadway Class

|  | Arterial Classification |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | I | II | III | IV |
| Range of Free-flow Speed | $45-55 \mathrm{MPH}$ | $35-45 \mathrm{MPH}$ | $30-35 \mathrm{MPH}$ | $25-35 \mathrm{MPH}$ |
| Typical Free Flow Speed | 50 MPH | 40 MPH | 33 MPH | 30 MPH |
| Level of Service |  | Speed (MPH) |  |  |
| A | $>42$ | $>35$ | $>30$ | $>25$ |
| B | $>34$ | $>28$ | $>24$ | $>19$ |
| C | $>27$ | $>22$ | $>18$ | $>13$ |
| D | $>21$ | $>17$ | $>10$ | $>=10$ |

## 3 Benefit Cost Analysis

To determine whether the completed signal retiming process benefits would outweigh the implementation costs, a Benefit Cost analysis will be performed using the input parameters collected during the travel time and delay studies conducted before and after the implementation of retiming plans. Some of the direct benefits of signal retiming include fuel savings, reduced delays \& stops, improved traffic flow, reduced toxic emissions \& improved air quality, reduced response time for emergency vehicles, etc. In addition, numerous indirect benefits could be attributed to signal retiming such as postponing long-term capacity improvements, reduced driver frustration, attracting tourists with better air quality, etc.

The benefits of the improved signal plans are projected over three years using two peak hours of travel time, one during the morning peak hour and the other during the evening peak hour. The following paragraphs describe the overall procedure of Benefit-Cost analysis utilized for the signal retiming evaluation process.

### 3.1 Benefits

As the first step, the cost savings associated with various parameters that were improved because of the retiming process were identified. Benefits are defined in terms of annualized cost savings and were calculated based on reduction in travel times and fuel savings derived from the before and after travel time data. As the first step, the benefit input parameter (travel time [seconds/vehicle] and fuel consumption [gallons/vehicle]) was multiplied with the corresponding peak hour directional traffic volume for each peak hour and direction to obtain the total travel time (vehicle-hours) or fuel consumption (gallons) for one hour. These calculations were performed for the before and after scenarios and the differences were obtained for the AM and PM peak hours. Then these differences (total travel time and fuel consumption) were multiplied with the corresponding dollar value to obtain the time and fuel savings in dollars. The daily savings in dollars are obtained by adding the benefits for AM and PM peak hours. The yearly savings are obtained by applying the daily savings for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffic volumes.

The above-mentioned calculations are explained in the following paragraphs for an example project: US 17/92 between Osceola Parkway and Columbia Avenue.

### 3.1.1 Travel Time Cost Savings

The cost associated with the lost travel time is valued at $\$ 15.47$ per hour for the year 2007 based on the latest Urban Mobility Report published by Texas Transportation Institute. The Urban Mobility Report page containing the delay value is provided in Appendix B of this memorandum. For the purposes of this study, the 2007 value was grown at $3 \%$ per year to obtain a value of $\$ 16.90$ per hour for the year 2010.

Based on the calculations using the field travel time data and traffic volume data from the latest (year 2008) traffic counts, a total annual cost savings (two peak hours combined) of \$270,095.80 was obtained from reduction in travel time for the US 17-92 study corridor.

### 3.1.2 Fuel Cost Savings

The savings on fuel costs were also included as part of the benefits in the Benefit-Cost analysis. The fuel costs were determined as $\$ 2.98$ based on the current market fuel costs. Based on the calculations using the field fuel consumption data and traffic volume data from the latest (year 2008) traffic counts, a total annual cost savings (two peak hours combined) of \$7,849.32 was obtained from reduction in fuel consumption for the US 17-92 study corridor.

Combining the cost savings from travel time and fuel consumption, a total annual cost savings of $\$ 277,945.12$ was obtained for the US 17-92 study corridor.

### 3.2 Costs

The second step is to obtain the project implementation cost of the signal retiming process. These project costs were provided by the FDOT and are provided in Appendix C of this report for the study projects. The annualized implementation costs were calculated assuming three (3) years of service life for the improvement and a 7\% rate of return on investment as currently recommended by the Federal Highway Administration (FHWA).

The annualized total signal-retiming cost was determined as $\$ 6,439.77$ from a one-time implementation cost of $\$ 16,900.00$ for the US 17-92 study corridor.

Tables 3 and 4 summarize the Measures of Effectiveness (MOEs) including travel time, delay, average speed, and fuel consumption for the through movement for the before and after scenarios, respectively during the AM and PM peak periods. Table 4 shows the benefits, costs, and Benefit-Cost ratio for the example study corridor.

Table 3: Summary of Before Study MOEs: US 17-92 between Osceola Parkway and Columbia Avenue

| Traffic Volume | MOE's per Vehicle |  |  |  | MOEs for all Vehicles |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Travel Time (sec/vehicle) | Delay (sec/vehicle) | Average <br> Speed <br> (mph) | Fuel Consumption (gallons/vehicle) | Total Travel Time (Vehicle-hour) | Total Fuel Consumption (gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 813 | 243.0 | 51.6 | 29.5 | 0.0700 | 54.88 | 56.91 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 904 | 269.4 | 70.2 | 26.6 | 0.0700 | 67.65 | 63.28 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1117 | 238.2 | 38.4 | 32.8 | 0.0830 | 73.91 | 92.71 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1708 | 261.0 | 52.2 | 29.9 | 0.0840 | 123.83 | 143.47 |

Table 4: Summary of After Study MOEs: US 17-92 between Osceola Parkway and Columbia Avenue

| Traffic Volume | MOE's Per Vehicle |  |  |  | MOEs for all Vehicles |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Travel Time (sec/vehicle) | Delay (sec/vehicle) | Average Speed (mph) | Fuel Consumption (gallons/vehicle) | Total Travel Time (Vehicle-hour) | Total Fuel Consumption (gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 813 | 179.4 | 15.6 | 39.9 | 0.0670 | 40.51 | 54.47 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 904 | 218.4 | 45.6 | 32.8 | 0.0680 | 54.84 | 61.47 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1117 | 208.2 | 18.6 | 37.5 | 0.0820 | 64.60 | 91.59 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1708 | 225.6 | 35.4 | 34.6 | 0.0820 | 107.03 | 140.06 |

Table 5: Summary of MOEs \& Benefit Cost Analysis: US 17-92 between Osceola Parkway and Columbia Avenue

| MOE | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 128.79 | 105.11 | 191.48 | 161.88 |
| Total Fuel Consumption (gallons) | 149.62 | 146.07 | 206.75 | 201.53 |
| BENEFITS | AM PEAK HOUR |  | PM PEAK HOUR |  |
| User Benefit Per Day | \$410.64 |  | \$515.84 |  |
| Annual User Benefit | \$123,192.72 |  | \$154,752.40 |  |
| Total Annual User Benefit | \$277,945.12 |  |  |  |
| Total Signal Retiming Annual Cost | \$6,439.77 |  |  |  |
| User Benefit / Cost Ratio | 43.16 |  |  |  |

## Notes:

1. Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown $3 \%$ per year)
2. Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
3. Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffic volumes.
4. The service life of the improvement was kept as three (3) years.
5. Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

### 3.3 Benefit-Cost Ratio

As shown in Table 5, a benefit cost ratio of 43.16 (greater than 1.0) was derived from the analysis for US 17-92 study corridor. The strong ratio indicates that the funds spent by FDOT/Metroplan Orlando to increase the operational capacity of the study corridor on US 17-92 between Osceola Parkway and Columbia Avenue in Osceola County receive approximately fortythree times in benefits derived through reduced costs associated with reduced travel time and fuel consumption. Therefore, the positive results of this benefit cost analysis justify the implementation of the recently completed signal timing improvements on this study corridor.

Similar to the MOE calculations and summaries shown in Tables 3 through 5, summary tables for each study roadway are provided in Appendix A of this report.

## 4 Conclusions

This chapter presents the conclusions derived from the TTD study results and a summary of Benefit-Cost ratio analysis results. GMB has conducted before and after travel time and delay studies on twenty-one (21) study roadways in the tri-county area (Orange, Seminole, and Osceola) of the Central Florida region to evaluate the benefits of the recently completed signal retiming projects on these roadways.

### 4.1 Travel Time and Delay Study

As part of the current study, various roadway characteristics and MOEs based on the travel time studies were summarized and provided in both tabular, and GIS map format for the study roadway segments. A total length of approximately 55 centerline miles of roadway segments was evaluated in this study. A summary showing the roadway miles that operate at LOS F in the before scenario (before the signal retiming) and in the after scenario (after the signal retiming) is provided in Table 6.

Table 6: Summary of Roadway Miles operating at LOS F

| Direction-Peak Hour | Before Scenario <br> $\%$ (Miles) | After Scenario <br> $\%$ (Miles) |
| :---: | :---: | :---: |
| NB/EB -AM | $3.13 \%(1.7)$ | $1.67 \%(1.0)$ |
| NB/EB - PM | $10.06 \%(5.6)$ | $6.05 \%(3.4)$ |
| SB/WB - AM | $8.98 \%(5.0)$ | $3.76 \%(2.1)$ |
| SB/WB - PM | $12.78 \%(7.1)$ | $7.05 \%(3.9)$ |
| Total | $19.4(35 \%)$ | $10.4(18 \%)$ |

As shown in Table 6, while approximately 35\% of the total roadway centerline miles were found to operate at LOS F before the implementation of the improved signal timings, only $18 \%$ of the total roadway centerline miles were found to operate at LOS F after the signal retiming projects were completed. In conclusion, LOS conditions on approximately $17 \%$ of the total roadway centerline miles were improved because of the recently completed signal retiming projects.

### 4.2 Benefit-Cost Ratio Analysis

As part of the current study, benefit-cost ratios were calculated for the 21 study roadways falling within the Central Florida region. Tables 7 through 10 illustrate the benefit-cost ratios by jurisdiction. Table 7 lists ratios for Seminole County, Table 8 lists ratios for Orange County, Table 9 lists ratios for the City of Orlando, and Table 10 lists the ratios for Osceola County.

Table 7: Benefit-Cost Ratio Summary for Seminole County Roadways

| Roadway | Limits | Annual <br> Benefit | Annual <br> Cost | B/C Ratio |
| :---: | :---: | :---: | :---: | :---: |
| US 17/92 | SR 46 (1st St) to 3rd St | \$91,803.70 | \$3,905.78 | 23.50 |
| SR 434 | Sand Lake Rd to Jamestown Blvd | \$97,494.71 | \$3,861.20 | 25.25 |
| SR 434 | Tollgate Trail to Wayman St | \$872,118.48 | \$11,926.92 | 73.12 |
| SR 434 | Mitchell Hammock Rd to Palm Valley Dr | \$486,025.44 | \$9,678.71 | 50.22 |
| SR 436 | Wilshire Dr to Casselton Dr | \$933,847.95 | \$17,509.32 | 53.33 |
| SR 46 | Park Dr to Sanford Ave | \$115,608.62 | \$3,905.78 | 29.60 |
| CR 46A | SR 417 NB Ramp to SR 417 SB Ramp | \$50,435.76 | \$3,861.20 | 13.06 |
| SR 426 | Old Howell Branch Rd to Dean Rd | \$380,396.32 | \$13,717.86 | 27.73 |
| Red Bug Lake Rd | SR 417 EB to SR 417 WB | \$160,113.12 | \$3,861.20 | 41.47 |

Table 8: Benefit-Cost Ratio Summary for Orange County Roadways

| Street | Limits | Annual <br> Benefit | Annual <br> Cost | B/C Ratio |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Alafaya Tl (SR 434) | Challenger Pkwy Intersection | $\$ 513,383.90$ | $\$ 1,566.12$ | 327.81 |
| Aloma Ave (SR 426) | Phelps Ave to Palmetto Ave | $\$ 953,161.33$ | $\$ 12,879.55$ | 74.01 |
| Colonial Dr E. (SR 50) | Murdock Blvd to Avalon Park Blvd | $\$ 1,490,036.19$ | $\$ 17,219.72$ | 86.53 |
| Conway Rd (SR 15) | Michigan St to Hoffner Ave | $\$ 494,870.99$ | $\$ 10,402.71$ | 47.57 |
| Goldenrod Rd (SR 551) | Bates Rd to Charlin Pkwy | $\$ 795,728.03$ | $\$ 15,889.85$ | 50.08 |
| Orange Ave (SR 527) | Drennan St to Nela Ave | $\$ 1,132,333.09$ | $\$ 16,309.01$ | 69.43 |
| Semoran Blvd (SR 436) | Aloma Ave to Baldwin Park St | $\$ 605,321.47$ | $\$ 8,345.03$ | $\mathbf{7 2 . 5 4}$ |

Table 9: Benefit-Cost Ratio Summary for City of Orlando Roadways

| Street | Limits | Annual <br> Benefit | Annual <br> Cost | B/C Ratio |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Curry Ford Rd (SR 552) | Conway Rd to Woodgate Blvd | $\$ 758,784.55$ | $\$ 10,402.71$ | 72.94 |
| Semoran Blvd (SR 436) | Dahlia Dr to T.G. Lee Blvd | $\$ 1,609,694.32$ | $\$ 29,036.14$ | 55.44 |
| Colonial $\operatorname{Dr}$ (SR 50) | Mills Ave to Old Cheney Hwy | $\$ 917,396.80$ | $\$ 21,948.58$ | 41.8 |

## Table 10: Benefit-Cost Ratio Summary for Osceola County Roadways

| Street | Limits | Annual <br> Benefit | Annual <br> Cost | B/C Ratio |
| :--- | :--- | :--- | :---: | :---: |
| US 17/92 | Osceola Pkwy to Columbia Ave | $\$ 277,945.12$ | $\$ 6,439.77$ | 43.16 |
| US 192 | Denn John Ln to Turnpike NB Ramp | $\$ 392,859.77$ | $\$ 11,660.18$ | 33.69 |

As shown in Table 7, the benefit-cost ratios range between 13 and 73 for the signal retiming projects on Seminole County roadways. From Table 8, the benefit-cost ratios range between 38 and 328 for the signal retiming projects on Orange County roadways. As shown in Table 9, the benefit-cost ratios range between 42 and 73 for the signal retiming projects on the City of Orlando roadways. As shown in Table 10, the benefit-cost ratios are 39 and 43 for the two (2) signal retiming projects on Osceola County roadways.

In conclusion, all the twenty-one (21) study signal-retiming projects have benefit-cost ratios of greater than one (1). This means that the cost benefits derived from reduced travel time and fuel consumption exceeded the costs incurred from implementing improved signal timing plans on the study roadways. Therefore, these traffic operational improvements are well justified.

In addition, a summary of the annual travel time and fuel savings are shown in Table 11 for the study roadways. As shown in Table 11, 762,022 vehicle-hours of travel time are estimated to be saved with the improved signal timings on the study roadways. Similarly, the new improved signal timings could save $\mathbf{8 4 , 2 8 9}$ gallons of fuel.

Table 11: Annual Travel Time and Cost Savings Summary

| Roadway Name | Limits | Annual Time <br> Savings (vehicle hours) | Annual Fuel Savings (gallons) |
| :---: | :---: | :---: | :---: |
| Alafaya Trail | Challenger Pkwy Intersection | 29,747 | 3,580 |
| Aloma Ave | Balfour Dr to Palmetto Ave | 55,695 | 3,997 |
| Conway Rd | Michigan Ave to Hoffner Ave | 28,900 | 2,168 |
| CR 46A | SR 417 NB Ramp to SR 417 SB Ramp | 2,941 | 248 |
| Curry Ford Rd | Conway Rd to Woodgate Blvd | 44,537 | 2,048 |
| Goldenrod Rd | Bates Rd to Charlin Pkwy | 41,965 | 29,033 |
| Orange Ave | Drennan St to Nela Ave | 66,660 | 1,937 |
| Red Bug Lake Rd | SR 417 EB to SR 417 WB | 9,425 | 279 |
| SR 426 | Old Howell Branch Rd to Dean Rd | 22,050 | 2,603 |
| SR 434 | Sand Lake Rd to Jamestown Blvd | 5,627 | 804 |
| SR 434 | Tollgate Trail to Wayman St | 50,945 | 3,740 |
| SR 434 | Mitchell Hammock Rd to Palm Valley Dr | 28,567 | 1,087 |
| SR 436 | Aloma Ave to Baldwin Park St | 35,151 | 3,781 |
| SR 436 | Wilshire Dr to Casselton Dr | 54,198 | 6,009 |
| SR 436 | Dahlia Dr to T.G. Lee Blvd | 94,372 | 4,971 |
| SR 46 | Park Dr to Sanford Ave | 6,811 | 170 |
| SR 50 | Mills Ave to Old Cheney Hwy | 53,560 | 4,106 |
| SR 50 | Murdock Blvd to Avalon Park Blvd | 86,449 | 9,749 |
| US 17-92 | SR 46 (1st St) to 3rd St | 5,385 | 266 |
| US 17-92 | Osceola Pkwy to Columbia Ave | 15,982 | 2,634 |
| US 192 | Denn John Ln to Turnpike NB Ramp | 23,056 | 1,079 |
|  | Total Savings | 762,022 | 84,289 |

## 5 Appendices

Appendix A: Before \& After Travel Time \& Delay Study Results, GIS Maps, MOE Summaries, and Benefit-Cost Ratio Calculation Sheets

Appendix B: Page from 2007 Urban Mobility Report
Appendix C: Signal Retiming Project Costs

## Appendix A

Before \& After Travel Time \& Delay Study Results, GIS Maps, MOE Summaries, and Benefit-Cost Ratio Calculation Sheets

## Alafaya TI @

## Challenger Pkwy Intersection

## TABLE 1

Year 2010 METROPLAN Orlando Travel Time Study
Alafaya Trail - Northbound Direction Summary - Before Condition


## TABLE 1

Year 2010 METROPLAN Orlando Travel Time Study
Alafaya Trail - Southbound Direction Summary - Before Condition


TABLE 1
Year 2010 METROPLAN Orlando Travel Time Study
Alafaya Trail - Northbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{array}{r} \text { Area } \\ \text { Type }^{1} \\ \hline \end{array}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & \text { (mph) } \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | Stop <br> Delay <br> (sec) | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Colonial Dr to Challenger Pkwy | Orange | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,270 | 15 | Signal | 39.6 | 0.0 | II | 39.1 | A | 0.87 |  |
| Challenger Pkwy to College Knights Ct | Orange | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,270 | 15 | Stop | 33.6 | 0.6 | 11 | 46.1 | A | 1.02 |  |
| TOTAL |  |  |  |  |  |  | 45 | 4,541 |  |  | 73.2 | 0.6 | II | 42.3 | A | 0.94 | 0.029 galveh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Colonial Dr to Challenger Pkwy | Orange | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,270 | 8 | Signal | 55.8 | 13.8 | II | 27.7 | c | 0.62 |  |
| Challenger Pkwy to College Knights Ct | Orange | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,270 | 8 | Stop | 35.4 | 1.2 | II | 43.7 | A | 0.97 |  |
| TOTAL |  |  |  |  |  |  | 45 | 4,541 |  |  | 91.2 | 15.0 | 11 | 33.9 | B | 0.75 | $0.029 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitio <br> 2. The Through lanes and Turn lanes are | ned from the la | st Orlando | Urban Area Transportation of travel. | tudy (OUA | S) Mode |  |  |  |  |  |  |  |  |  |  |  |  |

TABLE 1
Year 2010 METROPLAN Orlando Travel Time Study
Alafaya Trail - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{array}{r} \text { Area } \\ \text { Type }^{1} \\ \hline \end{array}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & \text { (mph) } \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | Stop <br> Delay <br> (sec) | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| College Knights Ct to Challenger Pkwy | Orange | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 2,270 | 17 | Signal | 37.2 | 0.6 | II | 41.6 | A | 0.92 |  |
| Challenger Pkwy to Colonial Dr | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,270 | 17 | Signal | 84.0 | 43.2 | 11 | 18.4 | D | 0.41 |  |
| TOTAL |  |  |  |  |  |  | 45 | 4,541 |  |  | 121.2 | 43.8 | II | 25.5 | C | 0.57 | 0.030 galven |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| College Knights Ct to Challenger Pkwy | Orange | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 2,270 | 7 | Signal | 75.0 | 27.0 | II | 20.6 | D | 0.46 |  |
| Challenger Pkwy to Colonial Dr | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,270 | 7 | Signal | 177.6 | 115.8 | II | 8.7 | F | 0.19 |  |
| TOTAL |  |  |  |  |  |  | 45 | 4,541 |  |  | 252.6 | 142.8 | 11 | 12.3 | F | 0.27 | $0.033 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitio <br> 2. The Through lanes and Turn lanes are | ned from the la | st Orlando | Urban Area Transportation of travel. | tudy (OUA | S) Mode |  |  |  |  |  |  |  |  |  |  |  |  |



Level of Services:


Travel Time Study
$\qquad$



Travel Time Study


## Alafaya Trail at Challenger Pkwy: Before \& After Study

Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 2391 | 76.8 | 3.6 | 40.3 | 0.0290 | 51.01 | 69.34 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2537 | 96.6 | 13.2 | 32.0 | 0.0300 | 68.08 | 76.11 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1668 | 148.8 | 68.4 | 20.8 | 0.0300 | 68.94 | 50.04 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1879 | 406.2 | 236.4 | 7.6 | 0.0380 | 212.01 | 71.40 |

*Traffic Volumes are obtained from the latest FDOT Counts

## Alafaya Trail at Challenger Pkwy: Before \& After Study

Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 2391 | 73.2 | 0.6 | 42.3 | 0.0290 | 48.62 | 69.34 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2537 | 91.2 | 15.0 | 33.9 | 0.0290 | 64.27 | 73.57 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1668 | 121.2 | 43.8 | 25.5 | 0.0300 | 56.16 | 50.04 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1879 | 252.6 | 142.8 | 12.3 | 0.0330 | 131.84 | 62.01 |

*Traffic Volumes are obtained from the latest FDOT Counts

## Alafaya Trail at Challenger Pkwy: Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 119.95 | 104.77 | 280.09 | 196.11 |
| Total Fuel Consumption (gallons) | 119.38 | 119.38 | 147.51 | 135.58 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 256.53$ | $\$ 1,454.75$ |
| Annual User Benefit | $\$ 76,957.53$ | $\$ 436,426.37$ |
| Total Annual User Benefit $=$ | $\$ 513,383.90$ |  |
| Total Signal Retiming Annual Cost | $\$ 1,566.12$ |  |
| User Benefit / Cost Ratio | 327.81 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of 7\% used by FDOT was used in arriving at the annual cost of improvements.


## Aloma Ave

## Phelps Ave to Palmetto Ave

TABLE 2
Year 2010 METROPLAN Orlando Travel Time Study
Aloma Avenue - Eastbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | RoadwayClass | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) ${ }^{\text {L }}$ LOS |  |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 739 | 5 | Signal | 27.6 | 8.4 | II | 18.3 | D | 0.52 |  |
| Phelps Ave to Lakemont Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 35 | 1,056 | 5 | Signal | 57.6 | 31.2 | II | 12.5 | F | 0.36 |  |
| Lakemont Ave to St. Andrews Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | Signal | 43.8 | 1.2 | II | 34.5 | B | 0.99 |  |
| St. Andrews Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,162 | 5 | Signal | 25.2 | 1.8 | 11 | 31.4 | B | 0.79 |  |
| Balfour Dr to Ranger Blva | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 2,006 | 5 | Signal | 38.4 | 1.2 | 11 | 35.6 | A | 0.89 |  |
| Ranger Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 1,320 | 5 | Signal | 73.8 | 45.0 | 11 | 12.2 | F | 0.30 |  |
| SR 436 to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 2,482 | 5 | Signal | 42.0 | 0.0 | 11 | 40.3 | A | 0.90 |  |
| Eastbrook Blvd to Forsyth Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 211 | 5 | Signal | 3.0 | 0.0 | 11 | 48.0 | A | 1.07 |  |
| Forsyth Rd to Palmetto Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 3,485 | 5 | Signal | 57.0 | 0.0 | II | 41.7 | A | 0.93 |  |
| TOTAL |  |  |  |  |  |  | 45 | 14,678 |  |  | 368.4 | 88.8 | 11 | 27.2 | C | 0.60 | 0.101 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 739 | 4 | Signal | 24.6 | 6.0 | 11 | 20.5 | D | 0.59 |  |
| Phelps Ave to Lakemont Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 35 | 1,056 | 4 | Signal | 64.8 | 34.2 | 11 | 11.1 | F | 0.32 |  |
| Lakemont Ave to St. Andrews Blva | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 4 | Signal | 58.2 | 9.0 | 11 | 26.0 | c | 0.74 |  |
| St. Andrews Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,162 | 4 | Signal | 28.8 | 1.2 | 11 | 27.5 | C | 0.69 |  |
| Balfour Dr to Ranger Blva | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 2,006 | 4 | Signal | 45.6 | 3.6 | 11 | 30.0 | B | 0.75 |  |
| Ranger Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 1,320 | 4 | Signal | 127.2 | 85.2 | 11 | 7.1 | F | 0.18 |  |
| SR 436 to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 2,482 | 4 | Signal | 70.8 | 15.6 | 11 | 23.9 | C | 0.53 |  |
| Eastbrook Blvd to Forsyth Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 211 | 4 | Signal | 3.6 | 0.0 | 11 | 40.0 | A | 0.89 |  |
| Forsyth Rd to Palmetto Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 3,485 | 4 | Signal | 146.4 | 58.8 | II | 16.2 | E | 0.36 |  |
| TOTAL |  |  |  |  |  |  | 45 | 14,678 |  |  | 570.0 | 213.6 | II | 17.6 | D | 0.39 | $0.107 \mathrm{gal/veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 2

Year 2010 METROPLAN Orlando Travel Time Study
Aloma Avenue - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & \text { (mph) } \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel Time (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | RoadwayClass | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Palmetto Ave | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 686 | 5 | Signal | 24.6 | 8.4 | II | 19.0 | D | 0.42 |  |
| Palmetto Ave to Forsyth Rd | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 3,485 | 5 | Signal | 88.8 | 15.0 | II | 26.8 | C | 0.59 |  |
| Forsyth Rd to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 45 | 211 | 5 | Signal | 10.2 | 4.2 | II | 14.1 | E | 0.31 |  |
| Eastbrook Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 2,482 | 5 | Signal | 153.6 | 81.6 | II | 11.0 | F | 0.28 |  |
| SR 436 to Ranger Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 25.8 | 0.0 | II | 34.9 | B | 0.87 |  |
| Ranger Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 5 | Signal | 62.4 | 14.4 | II | 21.9 | D | 0.55 |  |
| Balfour Dr to St. Andrews Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 1,162 | 5 | Signal | 36.6 | 7.2 | 11 | 21.6 | D | 0.62 |  |
| St. Andrews Blvd to Lakemont Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | Signal | 129.6 | 66.6 | II | 11.7 | F | 0.33 |  |
| Lakemont Ave to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,056 | 5 | Signal | 19.2 | 0.0 | II | 37.5 | A | 1.07 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,626 |  |  | 550.8 | 197.4 | II | 18.1 | D | 0.45 | 0.107 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Palmetto Ave | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 686 | 4 | Signal | 42.0 | 23.4 | II | 11.1 | F | 0.25 |  |
| Palmetto Ave to Forsyth Rd | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 3,485 | 4 | Signal | 82.8 | 17.4 | II | 28.7 | B | 0.64 |  |
| Forsyth Rd to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 45 | 211 | 4 | Signal | 3.6 | 0.0 | II | 40.0 | A | 0.89 |  |
| Eastbrook Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 2,482 | 4 | Signal | 78.6 | 25.8 | II | 21.5 | D | 0.54 |  |
| SR 436 to Ranger Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 4 | Signal | 24.0 | 0.0 | 11 | 37.5 | A | 0.94 |  |
| Ranger Blva to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 4 | Signal | 36.0 | 0.0 | II | 38.0 | A | 0.95 |  |
| Balfour Dr to St. Andrews Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 1,162 | 4 | Signal | 28.8 | 2.4 | 11 | 27.5 | C | 0.79 |  |
| St. Andrews Blvd to Lakemont Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 4 | Signal | 130.2 | 75.6 | 11 | 11.6 | F | 0.33 |  |
| Lakemont Ave to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,056 | 4 | Signal | 19.8 | 0.0 | II | 36.4 | A | 1.04 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,626 |  |  | 445.8 | 144.6 | II | 22.4 | c | 0.56 | $0.103 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type <br> 2. The Through lanes and Turn lane | ned from the la | st Orlando | Urban Area Transportation of travel. | udy (OUA | S) Model |  |  |  |  |  |  |  |  |  |  |  |  |

TABLE 2
Year 2010 METROPLAN Orlando Travel Time Study
Aloma Avenue - Eastbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Thru } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) ${ }^{\text {L }}$ LOS |  |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 739 | 5 | Signal | 21.6 | 6.0 | II | 23.3 | c | 0.67 |  |
| Phelps Ave to Lakemont Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 35 | 1,056 | 5 | Signal | 45.6 | 23.4 | II | 15.8 | E | 0.45 |  |
| Lakemont Ave to St. Andrews Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | signal | 37.8 | 1.2 | II | 40.0 | A | 1.14 |  |
| St. Andrews Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,162 | 5 | Signal | 18.6 | 0.0 | 11 | 42.6 | A | 1.06 |  |
| Balfour Dr to Ranger Blva | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 2,006 | 5 | Signal | 40.8 | 4.2 | 11 | 33.5 | B | 0.84 |  |
| Ranger Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 1,320 | 5 | Signal | 66.0 | 33.6 | 11 | 13.6 | E | 0.34 |  |
| SR 436 to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 2,482 | 5 | Signal | 37.8 | 0.0 | 11 | 44.8 | A | 0.99 |  |
| Eastbrook Blvd to Forsyth Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 211 | 5 | Signal | 4.8 | 0.0 | 11 | 30.0 | B | 0.67 |  |
| Forsyth Rd to Palmetto Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 3,485 | 5 | Signal | 48.0 | 0.0 | II | 49.5 | A | 1.10 |  |
| TOTAL |  |  |  |  |  |  | 45 | 14,678 |  |  | 321.0 | 68.4 | 11 | 31.2 | B | 0.69 | $0.101 \mathrm{gal/veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 739 | 5 | Signal | 28.8 | 10.2 | 11 | 17.5 | D | 0.50 |  |
| Phelps Ave to Lakemont Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 35 | 1,056 | 5 | Signal | 39.6 | 9.6 | II | 18.2 | D | 0.52 |  |
| Lakemont Ave to St. Andrews Blva | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | Signal | 45.6 | 3.6 | 11 | 33.2 | B | 0.95 |  |
| St. Andrews Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,162 | 5 | Signal | 26.4 | 4.2 | 11 | 30.0 | B | 0.75 |  |
| Balfour Dr to Ranger Blvd | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 2,006 | 5 | Signal | 55.8 | 14.4 | 11 | 24.5 | C | 0.61 |  |
| Ranger Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 1,320 | 5 | Signal | 74.4 | 40.2 | 11 | 12.1 | F | 0.30 |  |
| SR 436 to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 2,482 | 5 | Signal | 57.0 | 2.4 | 11 | 29.7 | B | 0.66 |  |
| Eastbrook Blvd to Forsyth Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 211 | 5 | Signal | 3.6 | 0.0 | 11 | 40.0 | A | 0.89 |  |
| Forsyth Rd to Palmetto Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 3,485 | 5 | Signal | 108.0 | 33.0 | II | 22.0 | D | 0.49 |  |
| total |  |  |  |  |  |  | 45 | 14,678 |  |  | 439.2 | 117.6 | II | 22.8 | c | 0.51 | $0.104 \mathrm{gal/veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lan | pproac | directi | el. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Year 2010 METROPLAN Orlando Travel Time Study
Aloma Avenue - Westbound Direction Summary - After Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{array}{c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ \text { (mph) } \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Palmetto Ave | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 686 | 5 | Signal | 18.0 | 5.4 | II | 26.0 | C | 0.58 |  |
| Palmetto Ave to Forsyth Rd | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 3,485 | 5 | Signal | 68.4 | 8.4 | II | 34.7 | B | 0.77 |  |
| Forsyth Rd to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 45 | 211 | 5 | Signal | 3.0 | 0.0 | II | 48.0 | A | 1.07 |  |
| Eastbrook Blva to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 2,482 | 5 | Signal | 138.0 | 67.2 | II | 12.3 | F | 0.31 |  |
| SR 436 to Ranger Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 22.8 | 0.0 | 11 | 39.5 | A | 0.99 |  |
| Ranger Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 5 | Signal | 33.6 | 0.0 | 11 | 40.7 | A | 1.02 |  |
| Balfour Dr to St. Andrews Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 1,162 | 5 | Signal | 28.8 | 1.2 | 11 | 27.5 | C | 0.79 |  |
| St. Andrews Blvd to Lakemont Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | Signal | 118.8 | 58.2 | II | 12.7 | F | 0.36 |  |
| Lakemont Ave to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,056 | 5 | Signal | 16.8 | 0.0 | II | 42.9 | A | 1.22 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,626 |  |  | 448.2 | 140.4 | II | 22.2 | C | 0.56 | 0.105 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Palmetto Ave | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 686 | 5 | Signal | 22.8 | 6.6 | 11 | 20.5 | D | 0.46 |  |
| Palmetto Ave to Forsyth Rd | Orange | Arterial | Outlying Business District | 2 | 2 | 0 | 45 | 3,485 | 5 | Signal | 88.8 | 24.0 | 11 | 26.8 | C | 0.59 |  |
| Forsyth Rd to Eastbrook Blvd | Orange | Arterial | Outlying Business District | 0 | 2 | 0 | 45 | 211 | 5 | Signal | 4.2 | 0.0 | II | 34.3 | B | 0.76 |  |
| Eastbrook Blvd to SR 436 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 40 | 2,482 | 5 | Signal | 94.8 | 41.4 | 11 | 17.8 | D | 0.45 |  |
| SR 436 to Ranger Blvd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 24.6 | 0.0 | II | 36.6 | A | 0.91 |  |
| Ranger Blvd to Balfour Dr | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 5 | Signal | 33.0 | 0.0 | II | 41.5 | A | 1.04 |  |
| Balfour Dr to St. Andrews Blva | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 1,162 | 5 | Signal | 21.0 | 0.0 | II | 37.7 | A | 1.08 |  |
| St. Andrews Blvd to Lakemont Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,218 | 5 | Signal | 44.4 | 1.8 | 11 | 34.1 | B | 0.97 |  |
| Lakemont Ave to Phelps Ave | Orange | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,056 | 5 | Signal | 25.8 | 3.0 | II | 27.9 | C | 0.80 |  |
| total |  |  |  |  |  |  | 40 | 14,626 |  |  | 359.4 | 76.8 | II | 27.7 | c | 0.69 | $0.101 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type <br> 2. The Through lanes and Turn lane | ned from the la | st Orlando | Urban Area Transportation of travel. | udy (OUA | S) Model |  |  |  |  |  |  |  |  |  |  |  |  |

## Aloma Avenue

- AM Peak


## Before Condition

Date of Collection: 4/22/2010 Distance: 2.77 miles From: Palmetto Ave To: Phelps Ave.

Start Time: 7:30 AM
End Time: 9:00 AM
EB Avg Speed: 27.2 MPH EB Travel Time: 6.14 MIN EB Delay Time: 1.48 MIN WB Avg Speed: 18.1 MPH WB Delay Time: 3.29 MIN


## Aloma Avenue

- AM Peak

After Condition
Date of Collection: 5/25/2010 Distance: 2.77 miles From: Palmetto Av
To: Phelps Ave.
Start Time: 7:30 AM
End Time: 9:00 AM
EB Avg Speed: 31.2 MPH B Travelime: 51.2 MPH EB Travel Time: 5.35 MIN

WB Avg Speed: 22.2 MPH WB Travel Time: 7.47 MIN WB Delay Time: 2.34 MIN

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Travel Time Study

## Aloma Avenue

- PM Peak


## Before Condition

Date of Collection: 4/22/2010 Distance: 2.77 miles From: Palmetto Ave To: Phelps Ave.

Start Time: 4:30 PM End Time: 6:00 PM

EB Avg Speed: 17.6 MPH EB Travel Time: 9.50 MIN EB Delay Time: 3.56 MIN WB Avg Speed: 22.4 MPH
WB Travel Time: 7.43 MIN WB Delay Time: 2.41 MIN WB Delay Time: 2.41 MIN


## Aloma Avenue

- PM Peak

After Condition
Date of Collection: 5/25/2010 Distance: 2.77 miles From: Palmetto Av To: Phelps Ave.
Start Time: 4:30 PM End Time: 6:00 PM

EB Avg Speed: 22.8 MPH EB Travel Time: 7.32 MIN EB Delay Time: 1.96 MIN

WB Avg Speed: 27.7 MPH WB Travel Time: 5.99 MIN WB Delay Time: 1.28 MIN



Aloma Ave: Phelps Ave to Palmetto Ave: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1148 | 368.4 | 88.8 | 27.2 | 0.1010 | 117.48 | 115.95 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2063 | 570.0 | 213.6 | 17.6 | 0.1070 | 326.64 | 220.74 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2216 | 550.8 | 197.4 | 18.1 | 0.1070 | 339.05 | 237.11 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1351 | 445.8 | 144.6 | 22.4 | 0.1030 | 167.30 | 139.15 |

*Traffic Volumes are obtained from the latest FDOT Counts

Aloma Ave: Phelps Ave to Palmetto Ave: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1148 | 321.0 | 68.4 | 31.2 | 0.1010 | 102.36 | 115.95 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2063 | 439.2 | 117.6 | 22.8 | 0.1040 | 251.69 | 214.55 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2216 | 448.2 | 140.4 | 22.2 | 0.1050 | 275.89 | 232.68 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1351 | 359.4 | 76.8 | 27.7 | 0.1010 | 134.87 | 136.45 |

*Traffic Volumes are obtained from the latest FDOT Counts

## Aloma Ave: Phelps Ave to Palmetto Ave: Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 456.53 | 378.26 | 493.94 | 386.56 |
| Total Fuel Consumption (gallons) | 353.06 | 348.63 | 359.89 | 351.00 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 1,335.99$ | $\$ 1,841.21$ |
| Annual User Benefit | $\$ 400,797.87$ | $\$ 552,363.46$ |
| Total Annual User Benefit $=$ | $\$ 953,161.33$ |  |
| Total Signal Retiming Annual Cost | $\$ 12,879.55$ |  |
| User Benefit / Cost Ratio | 74.01 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Colonial Dr E. (SR 50)

## Murdock Blvd to Avalon Park Blvd

## table 3

Year 2010 METROPLAN OrlandoTravel Time Study
SR 50 - Eastbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ |  | Thru Lanes ${ }^{2}$ | Right Turn Lanes ${ }^{2}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ <br> Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 7 | Signal | 24.6 | 5.4 | 1 | 24.9 | D | 0.55 |  |
| Murdock Blvd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 7 | Signal | 65.4 | 12.0 | 1 | 27.5 | C | 0.61 |  |
| Rouse Rd to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 7 | Signal | 65.4 | 21.0 | 1 | 23.7 | D | 0.53 |  |
| Walmart/Rouse Lake Rd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,957 | 7 | Signal | 94.8 | 42.6 | 1 | 21.3 | D | 0.47 |  |
| Alafya Trail to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 7 | Signal | 21.6 | 0.0 | 1 | 43.3 | A | 0.96 |  |
| Sophie Blvd to Woodbury Rd | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 2,746 | 7 | Signal | 45.0 | 4.8 | 1 | 41.6 | B | 0.92 |  |
| Woodbury Rd to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,373 | 7 | Signal | 19.8 | 0.0 | 1 | 47.3 | A | 1.05 |  |
| SR 408 to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 3 | 0 | 45 | 1,109 | 7 | Signal | 16.8 | 0.0 | 1 | 45.0 | A | 1.00 |  |
| Bonneville Dr to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,478 | 7 | Signal | 25.2 | 1.2 | 1 | 40.0 | B | 0.89 |  |
| Lake Pickett Rd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 2,693 | 7 | Signal | 49.2 | 4.8 | 1 | 37.3 | B | 0.68 |  |
| Pebble Beach Blvd to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 7 | Signal | 51.0 | 12.0 | 1 | 34.6 | B | 0.63 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,123 |  |  | 478.8 | 103.8 | 1 | 31.5 | C | 0.70 | $0.144 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 45.0 | 16.2 | 1 | 13.6 | F | 0.30 |  |
| Murdock Blvd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 6 | Signal | 130.2 | 67.8 | 1 | 13.8 | F | 0.31 |  |
| Rouse Rd to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 6 | Signal | 50.4 | 10.2 | 1 | 30.7 | c | 0.68 |  |
| Walmart/Rouse Lake Rd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,957 | 6 | Signal | 82.8 | 22.8 | 1 | 24.3 | D | 0.54 |  |
| Alafya Trail to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 6 | Signal | 38.4 | 10.8 | 1 | 24.4 | D | 0.54 |  |
| Sophie Blvd to Woodbury Rd | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 2,746 | 6 | Signal | 239.4 | 145.2 | 1 | 7.8 | F | 0.17 |  |
| Woodbury Rd to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,373 | 6 | Signal | 117.0 | 50.4 | 1 | 8.0 | F | 0.18 |  |
| SR 408 to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 3 | 0 | 45 | 1,109 | 6 | Signal | 58.2 | 16.8 | 1 | 13.0 | F | 0.29 |  |
| Bonneville Dr to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,478 | 6 | Signal | 73.2 | 27.6 | 1 | 13.8 | F | 0.31 |  |
| Lake Pickett Rd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 2,693 | 6 | Signal | 39.6 | 0.0 | 1 | 46.4 | A | 0.84 |  |
| Pebble Beach Blvd to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 6 | Signal | 61.2 | 13.8 | 1 | 28.8 | C | 0.52 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,123 |  |  | 935.4 | 381.6 | 1 | 16.1 | E | 0.36 | 0.161gal/veh |

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.
table 3
Year 2010 METROPLAN OrlandoTravel Time Study
SR 50 - Westbound Direction Summary - Before Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway <br> Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) LOS |  |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 845 | 7 | Signal | 89.4 | 60.0 | 1 | 6.4 | F | 0.12 |  |
| Avalon Park Blvd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 7 | Signal | 196.8 | 82.8 | 1 | 9.0 | F | 0.16 |  |
| Pebble Beach Blvd to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,693 | 7 | Signal | 189.6 | 81.0 | 1 | 9.7 | F | 0.22 |  |
| Lake Pickett Rd to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,478 | 7 | Signal | 45.6 | 6.6 | 1 | 22.1 | D | 0.49 |  |
| Bonneville Dr to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,109 | 7 | Signal | 21.0 | 1.2 | 1 | 36.0 | B | 0.80 |  |
| Sr 408 to Woodbury Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,373 | 7 | Signal | 25.2 | 3.0 | 1 | 37.1 | B | 0.83 |  |
| Woodbury Rd to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,746 | 7 | Signal | 69.6 | 20.4 | 1 | 26.9 | D | 0.60 |  |
| Sophie Blvd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,373 | 7 | Signal | 48.0 | 18.0 | 1 | 19.5 | E | 0.43 |  |
| Alafya Trail to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 7 | Signal | 46.8 | 0.0 | 1 | 43.1 | A | 0.96 |  |
| Walmart/Rouse Lake Rd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 7 | Signal | 37.8 | 0.0 | 1 | 41.0 | в | 0.91 |  |
| Rouse Rd to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 7 | Signal | 41.4 | 0.0 | 1 | 43.5 | A | 0.97 |  |
| total |  |  |  |  |  |  | 45 | 22,071 |  |  | 811.2 | 273.0 | 1 | 18.5 | E | 0.41 | 0.159 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 845 | 5 | Signal | 26.4 | 10.2 | 1 | 21.8 | D | 0.40 |  |
| Avalon Park Blvd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 5 | Signal | 38.4 | 0.0 | 1 | 45.9 | A | 0.84 |  |
| Pebble Beach Blvd to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,693 | 5 | Signal | 82.8 | 31.2 | 1 | 22.2 | D | 0.49 |  |
| Lake Pickett Rd to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,478 | 5 | Signal | 28.2 | 0.0 | 1 | 35.7 | B | 0.79 |  |
| Bonneville Dr to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,109 | 5 | Signal | 24.0 | 5.4 | 1 | 31.5 | C | 0.70 |  |
| Sr 408 to Woodbury Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,373 | 5 | Signal | 25.2 | 1.2 | 1 | 37.1 | B | 0.83 |  |
| Woodbury Rd to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,746 | 5 | Signal | 65.4 | 18.6 | 1 | 28.6 | C | 0.64 |  |
| Sophie Blvd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,373 | 5 | Signal | 76.8 | 42.6 | 1 | 12.2 | F | 0.27 |  |
| Alafya Trail to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 5 | Signal | 47.4 | 0.0 | 1 | 42.5 | A | 0.95 |  |
| Walmart/Rouse Lake Rd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 5 | Signal | 97.8 | 51.6 | 1 | 15.8 | F | 0.35 |  |
| Rouse Rd to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 5 | Signal | 51.6 | 7.2 | 1 | 34.9 | B | 0.78 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,071 |  |  | 564.0 | 168.0 | 1 | 26.7 | D | 0.59 | $0.145 \mathrm{gal} / \mathrm{veh}$ |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Note:1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 3

Year 2010 METROPLAN OrlandoTravel Time Study
SR 50 - Eastbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ |  | Thru Lanes ${ }^{2}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speedl | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 5 | Signal | 25.8 | 4.8 | 1 | 23.7 | D | 0.53 |  |
| Murdock Blvd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 5 | Signal | 60.6 | 16.2 | 1 | 29.7 | C | 0.66 |  |
| Rouse Rd to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 5 | Signal | 33.0 | 0.0 | 1 | 46.9 | A | 1.04 |  |
| Walmart/Rouse Lake Rd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,957 | 5 | Signal | 46.8 | 0.6 | 1 | 43.1 | A | 0.96 |  |
| Alafya Trail to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 5 | Signal | 19.8 | 0.0 | 1 | 47.3 | A | 1.05 |  |
| Sophie Blvd to Woodbury Rd | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 2,746 | 5 | Signal | 45.6 | 2.4 | 1 | 41.1 | B | 0.91 |  |
| Woodbury Rd to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,373 | 5 | Signal | 20.4 | 0.0 | 1 | 45.9 | A | 1.02 |  |
| SR 408 to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 3 | 0 | 45 | 1,109 | 5 | Signal | 15.6 | 0.0 | 1 | 48.5 | A | 1.08 |  |
| Bonneville Dr to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,478 | 5 | Signal | 21.0 | 0.0 | 1 | 48.0 | A | 1.07 |  |
| Lake Pickett Rd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 2,693 | 5 | Signal | 36.0 | 0.0 | 1 | 51.0 | A | 0.93 |  |
| Pebble Beach Blvd to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 5 | Signal | 48.0 | 4.2 | 1 | 36.7 | B | 0.67 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,123 |  |  | 372.6 | 28.2 | 1 | 40.5 | B | 0.90 | $0.142 \mathrm{ga} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 4 | Signal | 20.4 | 0.0 | 1 | 30.0 | c | 0.67 |  |
| Murdock Blvd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 4 | Signal | 65.4 | 12.6 | 1 | 27.5 | C | 0.61 |  |
| Rouse Rd to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 4 | Signal | 40.2 | 0.0 | 1 | 38.5 | B | 0.86 |  |
| Walmart/Rouse Lake Rd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,957 | 4 | Signal | 60.6 | 6.0 | 1 | 33.3 | C | 0.74 |  |
| Alafya Trail to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 4 | Signal | 24.0 | 0.0 | 1 | 39.0 | B | 0.87 |  |
| Sophie Blvd to Woodbury Rd | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 2,746 | 4 | Signal | 105.6 | 48.0 | 1 | 17.7 | E | 0.39 |  |
| Woodbury Rd to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,373 | 4 | Signal | 76.8 | 36.6 | 1 | 12.2 | F | 0.27 |  |
| SR 408 to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 3 | 0 | 45 | 1,109 | 4 | Signal | 53.4 | 16.2 | 1 | 14.2 | F | 0.31 |  |
| Bonneville Dr to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,478 | 4 | Signal | 60.0 | 22.2 | 1 | 16.8 | E | 0.37 |  |
| Lake Pickett Rd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 2,693 | 4 | Signal | 40.8 | 0.0 | 1 | 45.0 | A | 0.82 |  |
| Pebble Beach Blvd to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 4 | Signal | 53.4 | 6.6 | 1 | 33.0 | C | 0.60 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,123 |  |  | 600.6 | 148.2 | 1 | 25.1 | D | 0.56 | 0.150gal/veh |

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.
table 3
Year 2010 METROPLAN OrlandoTravel Time Study
SR 50 - Westbound Direction Summary - After Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right Turn Lanes ${ }^{2}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway <br> Class | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 845 | 5 | Signal | 68.4 | 47.4 | 1 | 8.4 | F | 0.15 |  |
| Avalon Park Blvd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 5 | Signal | 88.8 | 24.0 | 1 | 19.9 | E | 0.36 |  |
| Pebble Beach Blvd to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,693 | 5 | Signal | 177.0 | 80.4 | 1 | 10.4 | F | 0.23 |  |
| Lake Pickett Rd to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,478 | 5 | Signal | 45.6 | 10.2 | 1 | 22.1 | D | 0.49 |  |
| Bonneville Dr to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,109 | 5 | Signal | 18.0 | 0.0 | 1 | 42.0 | B | 0.93 |  |
| Sr 408 to Woodbury Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,373 | 5 | Signal | 20.4 | 0.0 | 1 | 45.9 | A | 1.02 |  |
| Woodbury Rd to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,746 | 5 | Signal | 54.0 | 7.8 | 1 | 34.7 | B | 0.77 |  |
| Sophie Blvd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,373 | 5 | Signal | 29.4 | 1.2 | 1 | 31.8 | C | 0.71 |  |
| Alafya Trail to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 5 | Signal | 45.0 | 0.0 | 1 | 44.8 | A | 1.00 |  |
| Walmart/Rouse Lake Rd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 5 | Signal | 48.0 | 0.6 | 1 | 32.2 | C | 0.72 |  |
| Rouse Rd to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 5 | Signal | 58.8 | 17.4 | 1 | 30.6 | c | 0.68 |  |
| total |  |  |  |  |  |  | 45 | 22,071 |  |  | 653.4 | 189.0 | 1 | 23.0 | D | 0.51 | $0.151 \mathrm{ga} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Avalon Park Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 55 | 845 | 4 | Signal | 24.0 | 8.4 | 1 | 24.0 | D | 0.44 |  |
| Avalon Park Blvd to Pebble Beach Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 55 | 2,587 | 4 | Signal | 39.0 | 0.0 | 1 | 45.2 | A | 0.82 |  |
| Pebble Beach Blvd to Lake Pickett Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,693 | 4 | Signal | 75.0 | 45.0 | 1 | 24.5 | D | 0.54 |  |
| Lake Pickett Rd to Bonneville Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,478 | 4 | Signal | 26.4 | 0.0 | 1 | 38.2 | B | 0.85 |  |
| Bonneville Dr to SR 408 | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,109 | 4 | Signal | 19.8 | 0.0 | 1 | 38.2 | B | 0.85 |  |
| Sr 408 to Woodbury Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,373 | 4 | Signal | 23.4 | 4.8 | 1 | 40.0 | B | 0.89 |  |
| Woodbury Rd to Sophie Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,746 | 4 | Signal | 54.0 | 13.2 | 1 | 34.7 | B | 0.77 |  |
| Sophie Blvd to Alafaya Trail | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,373 | 4 | Signal | 69.6 | 39.0 | 1 | 13.4 | F | 0.30 |  |
| Alafya Trail to Walmart/Rouse Lake Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 4 | Signal | 46.8 | 0.0 | 1 | 43.1 | A | 0.96 |  |
| Walmart/Rouse Lake Rd to Rouse Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,270 | 4 | Signal | 81.0 | 29.4 | 1 | 19.1 | E | 0.42 |  |
| Rouse Rd to Murdock Blvd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 2,640 | 4 | Signal | 51.0 | 8.4 | 1 | 35.3 | B | 0.78 |  |
| TOTAL |  |  |  |  |  |  | 45 | 22,071 |  |  | 510.0 | 148.2 | 1 | 29.5 | C | 0.66 | $0.146 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

SR 50 - AM Peak

## Before Condition

Date of Collection: 1/12/2010 Distance: 4.18 miles From: Avalon Park

Start Time: 7:45 AM
End Time: 9:00 AM
EB Avg Speed: 31.5 MPH EB Travel Time: 7.98 MIN WB Avg Speed: 18.5 MPH
WB Travel Time: 13.52 MIN WB Delay Time: 4.55 MIN

## SR 50

 - AM Peak
## After Condition

Date of Collection: 5/4/2010 Distance: 4.18 miles From: Avalon Park Blvd To: Murdock Blvd.

Start Time: 7:45 AM
B Avg Speed: 40.5 MPH B Avg Speed. 40.5 MIN EB Delay Time: 0.47 MIN

WB Avg Speed: 23.0 MPH WB Travel Time: 10.89 MIN WB Delay Time: 3.15 MIN


2010 MEIROPLAN ORLANDO
Travel Time Study
$\qquad$


SR 50 : Murdock Blvd to Avalon Park Blvd: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1047 | 478.8 | 103.8 | 31.5 | 0.1440 | 139.25 | 150.77 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1778 | 935.4 | 381.6 | 16.1 | 0.1610 | 461.98 | 286.26 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1554 | 811.2 | 273.0 | 18.5 | 0.1590 | 350.17 | 247.09 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1587 | 564.0 | 168.0 | 26.7 | 0.1450 | 248.63 | 230.12 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 50 : Murdock Blvd to Avalon Park Blvd: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1047 | 372.6 | 28.2 | 40.5 | 0.1420 | 108.36 | 148.67 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1778 | 600.6 | 148.2 | 25.1 | 0.1500 | 296.63 | 266.70 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1554 | 653.4 | 189.0 | 23.0 | 0.1510 | 282.05 | 234.65 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1587 | 510.0 | 148.2 | 29.5 | 0.1460 | 224.83 | 231.70 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 50 : Murdock Blvd to Avalon Park Blvd: Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 489.42 | 390.42 | 710.61 | 521.45 |
| Total Fuel Consumption (gallons) | 397.85 | 383.33 | 516.37 | 498.40 |


|  | BENEFITS | AM PEAK HOUR |
| :---: | :---: | :---: |
| PM PEAK HOUR |  |  |
| User Benefit Per Day | $\$ 1,716.45$ | $\$ 3,250.34$ |
| Annual User Benefit | $\$ 514,933.99$ | $\$ 975,102.20$ |
| Total Annual User Benefit $=$ | $\$ 1,490,036.19$ |  |
| Total Signal Retiming Annual Cost | $\$ 17,219.72$ |  |
| User Benefit / Cost Ratio | 86.53 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Conway Rd (SR 15)

## Michigan Ave to Hoffner Ave

## TABLE 4

Year 2010 METROPLAN Orlando Travel Time Study
Conway Road - Northbound Direction Summary - Before Condition

| Roadway <br> Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | Thru Lanes ${ }^{2}$ | Right <br> Turn <br> Lanes $^{2}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | TravelTime(sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 317 | 8 | Signal | 46.8 | 32.4 | II | 4.6 | F | 0.12 |  |
| Hoffner Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 8 | Signal | 18.6 | 0.0 | II | 34.8 | B | 0.87 |  |
| Shenandoah Elementary School to Gattin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 4,330 | 8 | Signal | 88.8 | 9.6 | II | 33.2 | B | 0.83 |  |
| Gatio Ave to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 8 | Signal | 49.2 | 1.2 | 11 | 36.6 | A | 0.91 |  |
| Anderson Rd to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 8 | Signal | 40.8 | 14.4 | II | 22.1 | C | 0.55 |  |
| Lake Margaret Dr to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 8 | Signal | 56.4 | 8.4 | II | 31.3 | B | 0.78 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,144 |  |  | 300.6 | 66.0 | II | 27.5 | C | 0.69 | 0.081 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 317 | 5 | Signal | 40.8 | 26.4 | II | 5.3 | F | 0.13 |  |
| Hoffner Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 5 | Signal | 18.6 | 0.0 | II | 34.8 | B | 0.87 |  |
| Shenandoah Elementary School to Gatiin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 4,330 | 5 | Signal | 123.0 | 35.4 | II | 24.0 | C | 0.60 |  |
| Gatlin Ave to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 5 | Signal | 47.4 | 0.0 | II | 38.0 | A | 0.95 |  |
| Anderson Rd to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 24.0 | 0.0 | 11 | 37.5 | A | 0.94 |  |
| Lake Margaret Dr to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 5 | Signal | 52.2 | 3.6 | 11 | 33.8 | B | 0.84 |  |
| total |  |  |  |  |  |  | 40 | 12,144 |  |  | 306.0 | 65.4 | II | 27.1 | C | 0.68 | $0.081 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 4

Year 2010 METROPLAN Orlando Travel Time Study
Conway Road - Southbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | Left <br> Turn <br> Lanes ${ }^{2}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right <br> Turn <br> Lanes ${ }^{2}$ | $\begin{aligned} & \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 8 | Signal | 43.8 | 25.8 | 11 | 9.0 | F | 0.23 |  |
| Michigan St to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 8 | Signal | 60.0 | 7.8 | II | 29.4 | B | 0.73 |  |
| Lake Margaret Dr to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 8 | Signal | 24.0 | 0.0 | II | 37.5 | A | 0.94 |  |
| Anderson Rd to Gatin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 8 | Signal | 51.0 | 1.2 | II | 35.3 | A | 0.88 |  |
| Gatio Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40/35 | 4,330 | 8 | Signal | 81.6 | 0.0 | II | 36.2 | A | 0.90 |  |
| Shenandoah Elementary School to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 8 | Signal | 50.4 | 25.2 | II | 12.9 | F | 0.32 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,408 |  |  | 310.8 | 60.0 | II | 27.2 | C | 0.68 | $0.085 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 5 | Signal | 48.6 | 27.0 | 11 | 8.1 | F | 0.20 |  |
| Michigan St to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 5 | Signal | 84.6 | 26.4 | II | 20.9 | D | 0.52 |  |
| Lake Margaret Dr to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 26.4 | 0.0 | II | 34.1 | B | 0.85 |  |
| Anderson Rd to Gatin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 5 | Signal | 77.4 | 17.4 | II | 23.3 | C | 0.58 |  |
| Gatio Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40/35 | 4,330 | 5 | Signal | 84.0 | 0.0 | II | 35.1 | A | 0.88 |  |
| Shenandoah Elementary School to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 5 | Signal | 66.0 | 43.2 | 11 | 9.8 | F | 0.25 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,408 |  |  | 387.0 | 114.0 | II | 21.9 | D | 0.55 | $0.086 \mathrm{gal} / \mathrm{veh}$ |

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 4

Year 2010 METROPLAN Orlando Travel Time Study
Conway Road - Northbound Direction Summary - After Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | Right Turn Lanes ${ }^{2}$ | $\begin{aligned} & \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Avg. Fuel <br> Speed Limit Consump. |  |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 317 | 8 | Signal | 54.6 | 36.0 | 11 | 4.0 | F | 0.10 |  |
| Hoffner Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 8 | Signal | 22.2 | 0.0 | II | 29.2 | B | 0.73 |  |
| Shenandoah Elementary School to Gatiin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 4,330 | 8 | Signal | 68.4 | 0.0 | II | 43.2 | A | 1.08 |  |
| Gatio Ave to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 8 | Signal | 41.4 | 0.0 | 11 | 43.5 | A | 1.09 |  |
| Anderson Rd to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 8 | Signal | 23.4 | 1.2 | II | 38.5 | A | 0.96 |  |
| Lake Margaret Dr to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 8 | Signal | 55.2 | 6.6 | II | 32.0 | B | 0.80 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,144 |  |  | 265.2 | 43.8 | II | 31.2 | B | 0.78 | $0.081 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 317 | 6 | Signal | 36.0 | 21.0 | II | 6.0 | F | 0.15 |  |
| Hoffner Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 6 | Signal | 18.0 | 0.0 | II | 36.0 | A | 0.90 |  |
| Shenandoah Elementary School to Gatiin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 4,330 | 6 | Signal | 72.6 | 2.4 | II | 40.7 | A | 1.02 |  |
| Gattin Ave to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 6 | Signal | 41.4 | 0.0 | II | 43.5 | A | 1.09 |  |
| Anderson Rd to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 6 | Signal | 38.4 | 12.6 | II | 23.4 | C | 0.59 |  |
| Lake Margaret Dr to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 6 | Signal | 47.4 | 3.0 | 11 | 37.2 | A | 0.93 |  |
| total |  |  |  |  |  |  | 40 | 12,144 |  |  | 253.8 | 39.0 | II | 32.6 | B | 0.82 | $0.081 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 4

Year 2010 METROPLAN Orlando Travel Time Study
Conway Road - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & (\mathrm{sec}) \end{aligned}$ | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 7 | Signal | 39.0 | 21.0 | 11 | 10.2 | F | 0.25 |  |
| Michigan St to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 7 | Signal | 49.8 | 5.4 | 11 | 35.4 | A | 0.89 |  |
| Lake Margaret Dr to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 7 | Signal | 21.0 | 0.0 | 11 | 42.9 | A | 1.07 |  |
| Anderson Rd to Gatin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 7 | Signal | 46.8 | 0.0 | II | 38.5 | A | 0.96 |  |
| Gatlin Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40/35 | 4,330 | 7 | Signal | 81.6 | 1.2 | II | 36.2 | A | 0.90 |  |
| Shenandoah Elementary School to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 7 | Signal | 38.4 | 21.0 | II | 16.9 | E | 0.42 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,408 |  |  | 276.6 | 48.6 | II | 30.6 | B | 0.76 | $0.083 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Michigan St | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 6 | Signal | 55.8 | 41.4 | II | 7.1 | F | 0.18 |  |
| Michigan St to Lake Margaret Dr | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,587 | 6 | Signal | 51.6 | 8.4 | II | 34.2 | B | 0.85 |  |
| Lake Margaret Dr to Anderson Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,320 | 6 | Signal | 22.2 | 1.2 | II | 40.5 | A | 1.01 |  |
| Anderson Rd to Gatin Ave | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40 | 2,640 | 6 | Signal | 55.2 | 9.0 | II | 32.6 | B | 0.82 |  |
| Gatiin Ave to Shenandoah Elementary School | Orange | Arterial | Residential Area | 1 | 2 | 0 | 40/35 | 4,330 | 6 | Signal | 66.0 | 0.0 | II | 44.7 | A | 1.12 |  |
| Shenandoah Elementary School to Hoffner Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 40 | 950 | 6 | Signal | 24.6 | 6.0 | II | 26.3 | C | 0.66 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,408 |  |  | 275.4 | 66.0 | 11 | 30.7 | B | 0.77 | $0.082 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




Travel Time Study
$\begin{array}{lll} & \\ 0 & 0.25 & 0.5\end{array}$

$0 \quad 0.25$ Miles

Conway Rd: Michigan Ave to Hoffner Ave: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1592 | 300.6 | 66.0 | 27.5 | 0.0810 | 132.93 | 128.95 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2151 | 306.0 | 65.4 | 27.1 | 0.0810 | 182.84 | 174.23 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1088 | 310.8 | 60.0 | 27.2 | 0.0850 | 93.93 | 92.48 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1263 | 387.0 | 114.0 | 21.9 | 0.0860 | 135.77 | 108.62 |

*Traffic Volumes are obtained from the latest Orange County Counts

## Conway Rd: Michigan Ave to Hoffner Ave: Before \& After Study

Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1592 | 265.2 | 43.8 | 31.2 | 0.0810 | 117.28 | 128.95 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2151 | 253.8 | 39.0 | 32.6 | 0.0810 | 151.65 | 174.23 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1088 | 276.6 | 48.6 | 30.6 | 0.0830 | 83.59 | 90.30 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1263 | 275.4 | 66.0 | 30.7 | 0.0820 | 96.62 | 103.57 |

*Traffic Volumes are obtained from the latest Orange County Counts

Conway Rd: Michigan Ave to Hoffner Ave: Before \& After Study
Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 226.86 | 200.87 | 318.61 | 248.27 |
| Total Fuel Consumption (gallons) | 221.43 | 219.26 | 282.85 | 277.80 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 445.73$ | $\$ 1,203.84$ |
| Annual User Benefit | $\$ 133,718.02$ | $\$ 361,152.96$ |
| Total Annual User Benefit $=$ | $\$ 494,870.99$ |  |
| Total Signal Retiming Annual Cost | $\$ 10,402.71$ |  |
| User Benefit / Cost Ratio | 47.57 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

Goldenrod Rd (SR 551)
Bates Rd to Charlin Pkwy

## TABLE 5

Year 2010 METROPLAN Orlando Travel Time Study
Goldenrod Road - Northbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance(ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & (\mathrm{sec}) \end{aligned}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | Roadway SegmentAverage Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | $\begin{array}{\|l\|} \hline \text { Avg Speed } \\ \text { Speed Limit } \end{array}$ | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 792 | 6 | Signal | 24.6 | 0.0 | 1 | 22.0 | D | 0.49 |  |
| Charlin Pkwy to Sun Vista Way | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,531 | 6 | Signal | 24.0 | 0.0 | 1 | 43.5 | A | 0.97 |  |
| Sun Vista Way to Pershing Ave | Orange | Arterial | Residential Area | 2 | 2 | 0 | 50 | 2,323 | 6 | Signal | 39.6 | 2.4 | 1 | 40.0 | B | 0.80 |  |
| Pershing Ave to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50/45 | 6,336 | 6 | Signal | 141.6 | 33.0 | 1 | 30.5 | c | 0.68 |  |
| Curry Ford Rd to Lake Underhill Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 6 | Signal | 282.0 | 96.6 | 1 | 21.6 | D | 0.48 |  |
| Lake Underhill Rd to SR 408 EB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 370 | 6 | Signal | 15.0 | 4.8 | 1 | 16.8 | E | 0.31 |  |
| SR 408 EB Ramp to SR 408 WB Ramp | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 370 | 6 | Signal | 7.2 | 0.0 | 1 | 35.0 | B | 0.70 |  |
| SR 408 WB Ramp to Valencia College Ln | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 4,541 | 6 | Signal | 84.0 | 10.8 | 1 | 36.9 | B | 0.82 |  |
| Valencia College Ln to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 5,280 | 6 | Signal | 156.6 | 60.0 | 1 | 23.0 | D | 0.51 |  |
| Colonial Dr to Liver Pool Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,587 | 6 | Signal | 42.0 | 0.0 | 1 | 42.0 | B | 0.93 |  |
| Liver Pool Blvd to Bates Rd | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 2,693 | 6 | Signal | 42.6 | 1.2 | 1 | 43.1 | A | 0.96 |  |
| TOTAL |  |  |  |  |  |  | 45 | 35,746 |  |  | 859.2 | 208.8 | 1 | 28.4 | C | 0.63 | $0.230 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 792 | 5 | Signal | 17.4 | 0.0 | 1 | 31.0 | C | 0.69 |  |
| Charlin Pkwy to Sun Vista Way | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,531 | 5 | Signal | 30.6 | 0.0 | 1 | 34.1 | B | 0.76 |  |
| Sun Vista Way to Pershing Ave | Orange | Arterial | Residential Area | 2 | 2 | 0 | 50 | 2,323 | 5 | Signal | 59.4 | 16.2 | 1 | 26.7 | D | 0.53 |  |
| Pershing Ave to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50/45 | 6,336 | 5 | Signal | 178.2 | 59.4 | 1 | 24.2 | D | 0.54 |  |
| Curry Ford Rd to Lake Underhill Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 5 | Signal | 239.4 | 75.0 | 1 | 25.4 | D | 0.56 |  |
| Lake Underhill Rd to SR 408 EB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 370 | 5 | Signal | 9.0 | 0.0 | 1 | 28.0 | c | 0.51 |  |
| SR 408 EB Ramp to SR 408 WB Ramp | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 370 | 5 | Signal | 7.2 | 0.0 | 1 | 35.0 | B | 0.70 |  |
| SR 408 WB Ramp to Valencia College Ln | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 4,541 | 5 | Signal | 77.4 | 1.8 | 1 | 40.0 | B | 0.89 |  |
| Valencia College Ln to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 5,280 | 5 | Signal | 199.8 | 101.4 | 1 | 18.0 | E | 0.40 |  |
| Colonial Dr to Liver Pool Blva | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,587 | 5 | Signal | 55.8 | 7.2 | 1 | 31.6 | c | 0.70 |  |
| Liver Pool Blvd to Bates Rd | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 2,693 | 5 | Signal | 61.8 | 15.0 | 1 | 29.7 | c | 0.66 |  |
| TOTAL |  |  |  |  |  |  | 45 | 35,746 |  |  | 936.0 | 276.0 | 1 | 26.0 | D | 0.58 | $0.276 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 5

Year 2010 METROPLAN Orlando Travel Time Study
Goldenrod Road - Southbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | LeftTurnLanes $^{2}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | Right <br> Turn <br> Lanes $^{2}$ | Speed Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel Time (sec) | Stop <br> Delay (sec) | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Bates Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | Signal | 15.0 | 3.6 | 1 | 16.8 | E | 0.37 |  |
| Bates Rd to Liver Pool Blva | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 6 | Signal | 41.4 | 0.0 | 1 | 44.3 | A | 0.99 |  |
| Liver Pool Blvd to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,587 | 6 | Signal | 129.0 | 73.8 | 1 | 13.7 | F | 0.30 |  |
| Colonial Dr to Valencia College Ln | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 5,280 | 6 | Signal | 98.4 | 11.4 | 1 | 36.6 | B | 0.81 |  |
| Valencia College Ln to SR 408 WB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 4,541 | 6 | Signal | 104.4 | 28.8 | 1 | 29.7 | C | 0.66 |  |
| SR 408 WB Ramp to SR 408 EB Ramp | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | Signal | 16.8 | 6.6 | 1 | 15.0 | F | 0.33 |  |
| SR 408 EB Ramp to Lake Underhill Rd | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 370 | 6 | Signal | 10.8 | 1.2 | 1 | 23.3 | D | 0.52 |  |
| Lake Underhill Rd to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 6 | Signal | 203.4 | 44.4 | 1 | 29.9 | C | 0.66 |  |
| Curry Ford Rd to Pershing Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50 | 6,336 | 6 | Signal | 129.0 | 13.2 | 1 | 33.5 | C | 0.67 |  |
| Pershing Ave to Sun Vista Way | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 2,323 | 6 | Signal | 45.6 | 2.4 | 1 | 34.7 | B | 0.69 |  |
| Sun Vista Way to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 1,531 | 6 | Signal | 29.4 | 3.6 | 1 | 35.5 | B | 0.71 |  |
| TOTAL |  |  |  |  |  |  | 45 | 35,323 |  |  | 823.2 | 189.0 | 1 | 29.3 | C | 0.65 | $0.231 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Bates Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 5 | Signal | 29.4 | 5.4 | 1 | 8.6 | F | 0.19 |  |
| Bates Rd to Liver Pool Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 5 | Signal | 45.0 | 1.2 | 1 | 40.8 | B | 0.91 |  |
| Liver Pool Blva to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,587 | 5 | Signal | 110.4 | 52.8 | 1 | 16.0 | F | 0.36 |  |
| Colonial Dr to Valencia College Ln | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 5,280 | 5 | Signal | 102.6 | 11.4 | 1 | 35.1 | B | 0.78 |  |
| Valencia College Ln to SR 408 WB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 4,541 | 5 | Signal | 186.6 | 75.0 | 1 | 16.6 | E | 0.37 |  |
| SR 408 WB Ramp to SR 408 EB Ramp | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 5 | Signal | 20.4 | 9.6 | 1 | 12.4 | F | 0.27 |  |
| SR 408 EB Ramp to Lake Underhill Rd | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 370 | 5 | Signal | 33.0 | 22.2 | 1 | 7.6 | F | 0.17 |  |
| Lake Underhill Rd to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 5 | Signal | 291.0 | 109.8 | 1 | 20.9 | E | 0.46 |  |
| Curry Ford Rd to Pershing Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50 | 6,336 | 5 | Signal | 122.4 | 14.4 | 1 | 35.3 | B | 0.71 |  |
| Pershing Ave to Sun Vista Way | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 2,323 | 5 | Signal | 45.0 | 3.0 | 1 | 35.2 | B | 0.70 |  |
| Sun Vista Way to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 1,531 | 5 | Signal | 28.8 | 0.0 | 1 | 36.2 | B | 0.72 |  |
| total |  |  |  |  |  |  | 45 | 35,323 |  |  | 1,014.6 | 304.8 | 1 | 23.7 | D | 0.53 | $0.238 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lanes are p | for the approach | fthe direc |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 5

Year 2010 METROPLAN Orlando Travel Time Study
Goldenrod Road - Northbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | Segment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility |  | Turn |  | Turn |  | Distance |  | Control | Time | Delay | Roadway | Aver | Speed | Avg Speed 1 | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 792 | 5 | Signal | 11.4 | 0.0 | 1 | 47.4 | A | 1.05 |  |
| Charlin Pkwy to Sun Vista Way | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,531 | 5 | Signal | 27.0 | 2.4 | 1 | 38.7 | B | 0.86 |  |
| Sun Vista Way to Pershing Ave | Orange | Arterial | Residential Area | 2 | 2 | 0 | 50 | 2,323 | 5 | Signal | 51.6 | 14.4 | 1 | 30.7 | C | 0.61 |  |
| Pershing Ave to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50/45 | 6,336 | 5 | Signal | 120.0 | 14.4 | 1 | 36.0 | B | 0.80 |  |
| Curry Ford Rd to Lake Underhill Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 5 | Signal | 303.0 | 117.0 | 1 | 20.1 | E | 0.45 |  |
| Lake Underhill Rd to SR 408 EB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 370 | 5 | Signal | 10.2 | 0.0 | 1 | 24.7 | D | 0.55 |  |
| SR 408 EB Ramp to SR 408 WB Ramp | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 370 | 5 | Signal | 7.2 | 0.0 | 1 | 35.0 | B | 0.78 |  |
| SR 408 WB Ramp to Valencia College Ln | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 4,541 | 5 | Signal | 67.8 | 0.0 | 1 | 45.7 | A | 1.01 |  |
| Valencia College Ln to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 5,280 | 5 | Signal | 100.8 | 7.8 | 1 | 35.7 | B | 0.79 |  |
| Colonial Dr to Liver Pool Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,587 | 5 | Signal | 43.2 | 0.0 | 1 | 40.8 | B | 0.91 |  |
| Liver Pool Blvd to Bates Rd | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 2,693 | 5 | Signal | 40.8 | 0.0 | 1 | 45.0 | A | 1.00 |  |
| TOTAL |  |  |  |  |  |  | 45 | 35,746 |  |  | 783.0 | 156.0 | 1 | 31.1 | C | 0.69 | $0.221 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 792 | 6 | Signal | 18.0 | 0.0 | 1 | 30.0 | c | 0.67 |  |
| Charlin Pkwy to Sun Vista Way | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 1,531 | 6 | Signal | 22.8 | 0.0 | 1 | 45.8 | A | 1.02 |  |
| Sun Vista Way to Pershing Ave | Orange | Arterial | Residential Area | 2 | 2 | 0 | 50 | 2,323 | 6 | Signal | 42.6 | 2.4 | 1 | 37.2 | B | 0.74 |  |
| Pershing Ave to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50/45 | 6,336 | 6 | Signal | 276.0 | 128.4 | 1 | 15.7 | F | 0.35 |  |
| Curry Ford Rd to Lake Underhill Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 6 | Signal | 206.4 | 52.8 | 1 | 29.5 | C | 0.66 |  |
| Lake Underhill Rd to SR 408 EB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 370 | 6 | Signal | 9.0 | 0.0 | 1 | 28.0 | C | 0.62 |  |
| SR 408 EB Ramp to SR 408 WB Ramp | Orange | Arterial | Residential Area | 2 | 2 | 0 | 45 | 370 | 6 | Signal | 7.2 | 0.0 | 1 | 35.0 | B | 0.78 |  |
| SR 408 WB Ramp to Valencia College Ln | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 4,541 | 6 | Signal | 62.4 | 0.0 | 1 | 49.6 | A | 1.10 |  |
| Valencia College Ln to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 5,280 | 6 | Signal | 102.0 | 15.0 | 1 | 35.3 | B | 0.78 |  |
| Colonial Dr to Liver Pool Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,587 | 6 | Signal | 48.0 | 2.4 | 1 | 36.7 | B | 0.82 |  |
| Liver Pool Blvd to Bates Rd | Orange | Arterial | Residential Area | 0 | 2 | 0 | 45 | 2,693 | 6 | Signal | 46.8 | 3.0 | 1 | 39.2 | B | 0.87 |  |
| total |  |  |  |  |  |  | 45 | 35,746 |  |  | 841.2 | 204.0 | 1 | 29.0 | C | 0.64 | $0.233 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## table 5

Year 2010 METROPLAN Orlando Travel Time Study
Goldenrod Road - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs |  | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Bates Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | Signal | 4.8 | 0.0 | 1 | 52.5 | A | 1.17 |  |
| Bates Rd to Liver Pool Blva | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 6 | Signal | 51.0 | 7.8 | 1 | 36.0 | B | 0.80 |  |
| Liver Pool Blva to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,587 | 6 | Signal | 99.0 | 43.2 | 1 | 17.8 | E | 0.40 |  |
| Colonial Dr to Valencia College Ln | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 5,280 | 6 | Signal | 91.8 | 5.4 | 1 | 39.2 | B | 0.87 |  |
| Valencia College Ln to SR 408 WB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 4,541 | 6 | Signal | 110.4 | 25.8 | 1 | 28.0 | c | 0.62 |  |
| SR 408 WB Ramp to SR 408 EB Ramp | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | Signal | 16.2 | 3.6 | 1 | 15.6 | F | 0.35 |  |
| SR 408 EB Ramp to Lake Underhill Rd | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 370 | 6 | Signal | 9.0 | 0.0 | 1 | 28.0 | C | 0.62 |  |
| Lake Underhill Rd to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 6 | Signal | 166.2 | 12.6 | 1 | 36.6 | B | 0.81 |  |
| Curry Ford Rd to Pershing Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50 | 6,336 | 6 | Signal | 112.8 | 7.8 | 1 | 38.3 | B | 0.77 |  |
| Pershing Ave to Sun Vista Way | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 2,323 | 6 | Signal | 37.2 | 1.2 | 1 | 42.6 | A | 0.85 |  |
| Sun Vista Way to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 1,531 | 6 | Signal | 31.2 | 0.0 | 1 | 33.5 | c | 0.67 |  |
| TOTAL |  |  |  |  |  |  | 45 | 35,323 |  |  | 729.6 | 107.4 | 1 | 33.0 | c | 0.73 | $0.22 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Bates Rd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 4 | Signal | 7.2 | 0.0 | 1 | 35.0 | B | 0.78 |  |
| Bates Rd to Liver Pool Blvd | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 4 | Signal | 39.0 | 0.0 | 1 | 47.1 | A | 1.05 |  |
| Liver Pool Blva to Colonial Dr | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 2,587 | 4 | Signal | 111.0 | 63.6 | 1 | 15.9 | F | 0.35 |  |
| Colonial Dr to Valencia College Ln | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 5,280 | 4 | Signal | 87.6 | 6.6 | 1 | 41.1 | B | 0.91 |  |
| Valencia College Ln to SR 408 WB Ramp | Orange | Arterial | Residential Area | 0 | 2 | 1 | 45 | 4,541 | 4 | Signal | 312.6 | 168.0 | 1 | 9.9 | F | 0.22 |  |
| SR 408 WB Ramp to SR 408 EB Ramp | Orange | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 4 | Signal | 21.0 | 9.0 | 1 | 12.0 | F | 0.27 |  |
| SR 408 EB Ramp to Lake Underhill Rd | Orange | Arterial | Residential Area | 2 | 2 | 1 | 45 | 370 | 4 | Signal | 8.4 | 0.0 | 1 | 30.0 | c | 0.67 |  |
| Lake Underhill Rd to Curry Ford Rd | Orange | Arterial | Residential Area | 1 | 2 | 1 | 45 | 8,923 | 4 | Signal | 164.4 | 15.6 | 1 | 37.0 | B | 0.82 |  |
| Curry Ford Rd to Pershing Ave | Orange | Arterial | Residential Area | 1 | 2 | 1 | 50 | 6,336 | 4 | Signal | 118.8 | 18.0 | 1 | 36.4 | B | 0.73 |  |
| Pershing Ave to Sun Vista Way | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 2,323 | 4 | Signal | 36.0 | 0.0 | 1 | 44.0 | A | 0.88 |  |
| Sun Vista Way to Charlin Pkwy | Orange | Arterial | Residential Area | 1 | 2 | 0 | 50 | 1,531 | 4 | Signal | 23.4 | 0.0 | 1 | 44.6 | A | 0.89 |  |
| total |  |  |  |  |  |  | 45 | 35,323 |  |  | 929.4 | 280.8 | 1 | 25.9 | D | 0.58 | $0.237 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3. The Right turn lane at Colonial Drive is newly added. There was no right turn lane in Before Condiiton. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



Level of Services:

$\qquad$ | D |
| :--- |
| E |
| F |

2010 MEIROPLAN ORLANDO
Travel Time Study


## Level of Services:

$\qquad$
$\qquad$
City Boundary
Water


Travel Time Study

Goldenrod Rd: Lake Underhill Rd to Charlin Pkwy: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1228 | 859.2 | 208.8 | 28.4 | 0.2300 | 293.08 | 282.44 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1588 | 936.0 | 276.0 | 26.0 | 0.2760 | 412.88 | 438.29 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1454 | 823.2 | 189.0 | 29.3 | 0.2310 | 332.48 | 335.87 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1448 | 1014.6 | 304.8 | 23.7 | 0.2380 | 408.09 | 344.62 |

*Traffic Volumes are obtained from the latest FDOT Counts

Goldenrod Rd: Lake Underhill Rd to Charlin Pkwy: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1228 | 783.0 | 156.0 | 31.1 | 0.2210 | 267.09 | 271.39 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1588 | 841.2 | 204.0 | 29.0 | 0.2330 | 371.06 | 370.00 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1454 | 729.6 | 107.4 | 33.0 | 0.2200 | 294.68 | 319.88 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1448 | 929.4 | 280.8 | 25.9 | 0.2370 | 373.83 | 343.18 |

*Traffic Volumes are obtained from the latest FDOT Counts

Goldenrod Rd: Lake Underhill Rd to Charlin Pkwy: Before \& After Study
Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 625.56 | 561.77 | 820.97 | 744.89 |
| Total Fuel Consumption (gallons) | 618.31 | 591.27 | 782.91 | 713.18 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 1,158.76$ | $\$ 1,493.67$ |
| Annual User Benefit | $\$ 347,628.22$ | $\$ 448,099.81$ |
| Total Annual User Benefit $=$ | $\$ 795,728.03$ |  |
| Total Signal Retiming Annual Cost | $\$ 15,889.85$ |  |
| User Benefit/ Cost Ratio | 50.08 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Orange Ave (SR 527)

## Drennen Rd to Nela Ave

TABLE 6
Year 2010 METROPLAN Orlando Travel Time Study
Orange Avenue - Northbound Direction Summary - Before Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{aligned} & \text { Area } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ \text { (mph) } \end{gathered}$ | Distance <br> (tt) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HoUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 581 | 6 | Signal | 46.2 | 17.4 | 11 | 8.6 | F | 0.19 |  |
| Nela Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 57.6 | 3.0 | 11 | 15.6 | E | 0.35 |  |
| Lancaster Rd to Fairlane Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,376 | 6 | Signal | 54.6 | 3.6 | II | 29.7 | B | 0.74 |  |
| Fairlane Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 264 | 6 | Signal | 9.0 | 1.2 | II | 20.0 | D | 0.50 |  |
| Oak Ridge Rd to Hoffner Ave | Orange | One Way | Residential Area | 0 | 2 | 1 | 40 | 845 | 6 | Signal | 24.6 | 2.4 | 11 | 23.4 | C | 0.59 |  |
| Hoffner Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 6 | Stop | 35.4 | 0.0 | II | 38.6 | A | 0.97 |  |
| Mary Jess Rd to Gatin Ave | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 5,069 | 6 | Signal | 145.2 | 43.8 | 11 | 23.8 | C | 0.60 |  |
| Gatin Ave to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 370 | 6 | Signal | 25.8 | 15.6 | 11 | 9.8 | F | 0.24 |  |
| Holden Ave to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 6 | Signal | 52.8 | 0.0 | II | 39.5 | A | 0.99 |  |
| TOTAL |  |  |  |  |  |  | 40 | 15,893 |  |  | 451.2 | 87.0 | 11 | 24.0 | C | 0.60 | $0.103 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 581 | 4 | Signal | 46.2 | 15.6 | 11 | 8.6 | F | 0.19 |  |
| Nela Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 4 | Signal | 57.6 | 0.6 | 11 | 15.6 | E | 0.35 |  |
| Lancaster Rd to Fairlane Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,376 | 4 | Signal | 43.2 | 0.0 | II | 37.5 | A | 0.94 |  |
| Fairlane Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 264 | 4 | Signal | 13.2 | 6.0 | II | 13.6 | E | 0.34 |  |
| Oak Ridge Rd to Hoffner Ave | Orange | One Way | Residential Area | 0 | 2 | 1 | 40 | 845 | 4 | Signal | 39.6 | 16.2 | 11 | 14.5 | E | 0.36 |  |
| Hoffner Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 4 | Stop | 34.8 | 0.0 | II | 39.3 | A | 0.98 |  |
| Mary Jess Rd to Gatio Ave | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 5,069 | 4 | Signal | 213.0 | 84.6 | II | 16.2 | E | 0.41 |  |
| Gatin Ave to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 370 | 4 | Signal | 43.8 | 30.0 | II | 5.8 | F | 0.14 |  |
| Holden Ave to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 4 | Signal | 55.8 | 2.4 | II | 37.4 | A | 0.94 |  |
| TOTAL |  |  |  |  |  |  | 40 | 15,893 |  |  | 547.2 | 155.4 | II | 19.8 | D | 0.50 | $0.106 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area typ <br> 2. The Through lanes and Turn | ned from the la | est Orlando | Urban Area Transportation of travel. | udy (OU | SS) Mode |  |  |  |  |  |  |  |  |  |  |  |  |

TABLE 6
Year 2010 METROPLAN Orlando Travel Time Study
Orange Avenue - Southbound Direction Summary - Before Condition

| Roadway <br> Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway <br> Class | Roadway Segment Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) LoS |  |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 528 | 7 | Signal | 14.4 | 0.0 | II | 25.0 | C | 0.62 |  |
| Drennen Rd to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 7 | Signal | 73.8 | 17.4 | 11 | 28.3 | B | 0.71 |  |
| Holden Ave to Gatin Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 7 | Signal | 6.6 | 0.0 | II | 32.7 | B | 0.82 |  |
| Gatiin Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 5,016 | 7 | Signal | 85.2 | 0.6 | 11 | 40.1 | A | 1.00 |  |
| Mary Jess Rd to Hoffner Ave | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 35 | 1,901 | 7 | Signal | 42.0 | 9.0 | 11 | 30.9 | B | 0.88 |  |
| Hoffner Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 898 | 7 | Signal | 19.2 | 0.0 | 11 | 31.9 | B | 0.80 |  |
| Oak Ridge Rd to Fairlane Ave | Orange | One Way | Outlying Business District | 0 | 2 | 0 | 40 | 317 | 7 | Stop | 6.0 | 0.0 | 11 | 36.0 | A | 0.90 |  |
| Fairlane Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 2,376 | 7 | Signal | 44.4 | 0.6 | 11 | 36.5 | A | 0.81 |  |
| Lancaster Rd to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 7 | Signal | 22.2 | 1.8 | II | 40.5 | A | 0.90 |  |
| TOTAL |  |  |  |  |  |  | 40 | 15,734 |  |  | 313.8 | 29.4 | II | 34.2 | B | 0.85 | 0.105 galven |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 528 | 4 | Signal | 26.4 | 8.4 | 11 | 13.6 | E | 0.34 |  |
| Drennen Rd to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 4 | Signal | 88.8 | 13.2 | II | 23.5 | C | 0.59 |  |
| Holden Ave to Gatin Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 4 | Signal | 8.4 | 0.0 | 11 | 25.7 | C | 0.64 |  |
| Gatiin Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 5,016 | 4 | Signal | 94.2 | 5.4 | 11 | 36.3 | A | 0.91 |  |
| Mary Jess Rd to Hoffner Ave | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 35 | 1,901 | 4 | Signal | 40.8 | 2.4 | 11 | 31.8 | B | 0.91 |  |
| Hoffner Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 898 | 4 | Signal | 18.6 | 0.0 | 11 | 32.9 | B | 0.82 |  |
| Oak Ridge Rd to Fairlane Ave | Orange | One Way | Outlying Business District | 0 | 2 | 0 | 40 | 317 | 4 | Stop | 5.4 | 0.0 | II | 40.0 | A | 1.00 |  |
| Fairlane Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 2,376 | 4 | Signal | 55.2 | 9.6 | 11 | 29.3 | B | 0.65 |  |
| Lancaster Rd to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 4 | Signal | 25.8 | 0.0 | II | 34.9 | B | 0.78 |  |
| total |  |  |  |  |  |  | 40 | 15,734 |  |  | 363.6 | 39.0 | 11 | 29.5 | B | 0.74 | $0.106 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type <br> 2. The Through lanes and Turn la | ined from the la | est Orlando | Urban Area Transportation of travel. | tudy (OU | TS) Mode |  |  |  |  |  |  |  |  |  |  |  |  |

TABLE 6
Year 2010 METROPLAN Orlando Travel Time Study
Orange Avenue - Northbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{gathered} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel Time $\qquad$ <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | RoadwayClass | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 581 | 7 | Signal | 8.5 | 3.0 | 11 | 46.6 | A | 1.04 |  |
| Nela Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 7 | Signal | 18.6 | 0.0 | 1 | 48.4 | A | 1.08 |  |
| Lancaster Rd to Fairlane Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,376 | 7 | Signal | 43.8 | 4.8 | II | 37.0 | A | 0.92 |  |
| Fairlane Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 264 | 7 | Signal | 7.2 | 0.6 | 11 | 25.0 | C | 0.62 |  |
| Oak Ridge Rd to Hoffner Ave | Orange | One Way | Residential Area | 0 | 2 | 1 | 40 | 845 | 7 | Signal | 31.2 | 12.6 | II | 18.5 | D | 0.46 |  |
| Hoffner Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 7 | Stop | 32.4 | 4.8 | II | 42.2 | A | 1.06 |  |
| Mary Jess Rd to Gatio Ave | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 5,069 | 7 | Signal | 105.6 | 16.2 | 11 | 32.7 | B | 0.82 |  |
| Gatin Ave to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 370 | 7 | Signal | 7.2 | 0.0 | II | 35.0 | B | 0.87 |  |
| Holden Ave to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 7 | Signal | 61.2 | 7.2 | II | 34.1 | B | 0.85 |  |
| TOTAL |  |  |  |  |  |  | 40 | 15,893 |  |  | 315.7 | 49.2 | II | 34.3 | B | 0.86 | 0.102 galven |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 581 | 5 | Signal | 9.2 | 3.0 | 11 | 43.0 | A | 0.96 |  |
| Nela Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 5 | Signal | 18.0 | 0.0 | II | 50.0 | A | 1.11 |  |
| Lancaster Rd to Fairlane Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 2,376 | 5 | Signal | 55.8 | 14.4 | II | 29.0 | B | 0.73 |  |
| Fairlane Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 264 | 5 | Signal | 6.6 | 1.8 | II | 27.3 | C | 0.68 |  |
| Oak Ridge Rd to Hoffner Ave | Orange | One Way | Residential Area | 0 | 2 | 1 | 40 | 845 | 5 | Signal | 17.4 | 0.0 | II | 33.1 | B | 0.83 |  |
| Hoffner Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 2,006 | 5 | Stop | 31.8 | 0.0 | II | 43.0 | A | 1.08 |  |
| Mary Jess Rd to Gatiin Ave | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 5,069 | 5 | Signal | 84.6 | 3.6 | II | 40.8 | A | 1.02 |  |
| Gatin Ave to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 370 | 5 | Signal | 7.8 | 0.0 | II | 32.3 | B | 0.81 |  |
| Holden Ave to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 5 | Signal | 50.4 | 0.0 | 11 | 41.4 | A | 1.04 |  |
| total |  |  |  |  |  |  | 40 | 15,893 |  |  | 281.6 | 22.8 | II | 38.5 | A | 0.96 | $0.105 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

TABLE 6
Year 2010 METROPLAN Orlando Travel Time Study
Orange Avenue - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway <br> Class | Roadway Segment Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 528 | 7 | Signal | 11.4 | 0.0 | II | 31.6 | B | 0.79 |  |
| Drennen Rd to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 7 | Signal | 61.8 | 16.8 | 11 | 33.8 | B | 0.84 |  |
| Holden Ave to Gatin Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 7 | Signal | 6.0 | 0.0 | II | 36.0 | A | 0.90 |  |
| Gatiin Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 5,016 | 7 | Signal | 75.6 | 0.6 | 11 | 45.2 | A | 1.13 |  |
| Mary Jess Rd to Hoffner Ave | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 35 | 1,901 | 7 | Signal | 45.0 | 10.8 | 11 | 28.8 | B | 0.82 |  |
| Hoffner Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 898 | 7 | Signal | 18.6 | 0.0 | 11 | 32.9 | B | 0.82 |  |
| Oak Ridge Rd to Fairlane Ave | Orange | One Way | Outlying Business District | 0 | 2 | 0 | 40 | 317 | 7 | Stop | 5.4 | 0.0 | 11 | 40.0 | A | 1.00 |  |
| Fairlane Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 2,376 | 7 | Signal | 39.0 | 6.0 | 11 | 41.5 | A | 0.92 |  |
| Lancaster Rd to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 7 | Signal | 23.4 | 0.0 | II | 38.5 | A | 0.85 |  |
| TOTAL |  |  |  |  |  |  | 40 | 15,734 |  |  | 286.2 | 34.2 | II | 37.5 | A | 0.94 | 0.103 galven |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Drennen Rd | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 528 | 5 | Signal | 11.4 | 0.0 | 11 | 31.6 | B | 0.79 |  |
| Drennen Rd to Holden Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 3,062 | 5 | Signal | 81.0 | 12.6 | II | 25.8 | C | 0.64 |  |
| Holden Ave to Gatin Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 5 | Signal | 7.2 | 0.0 | 11 | 30.0 | B | 0.75 |  |
| Gatin Ave to Mary Jess Rd | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 40 | 5,016 | 5 | Signal | 83.4 | 0.0 | 11 | 41.0 | A | 1.03 |  |
| Mary Jess Rd to Hoffner Ave | Orange | One Way | Outlying Business District | 1 | 2 | 0 | 35 | 1,901 | 5 | Signal | 33.6 | 6.0 | 11 | 38.6 | A | 1.10 |  |
| Hoffner Ave to Oak Ridge Rd | Orange | One Way | Outlying Business District | 0 | 2 | 1 | 40 | 898 | 5 | Signal | 17.4 | 0.0 | 11 | 35.2 | A | 0.88 |  |
| Oak Ridge Rd to Fairlane Ave | Orange | One Way | Outlying Business District | 0 | 2 | 0 | 40 | 317 | 5 | Stop | 6.0 | 0.0 | II | 36.0 | A | 0.90 |  |
| Fairlane Ave to Lancaster Rd | Orange | Arterial | Outlying Business District | 0 | 2 | 1 | 45 | 2,376 | 5 | Signal | 57.6 | 7.8 | 11 | 28.1 | B | 0.62 |  |
| Lancaster Rd to Nela Ave | Orange | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 5 | Signal | 21.0 | 0.0 | II | 42.9 | A | 0.95 |  |
| total |  |  |  |  |  |  | 40 | 15,734 |  |  | 318.6 | 26.4 | 11 | 33.7 | B | 0.84 | $0.106 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type <br> 2. The Through lanes and Turn la | ined from the la | est Orlando | Urban Area Transportation of travel. | tudy (OU | TS) Mode |  |  |  |  |  |  |  |  |  |  |  |  |




2010 MEIROPLAN ORLANDO
Travel Time Study
$\qquad$



2010 MEIROPLAN ORLANDO
Travel Time Study
$\qquad$

Orange Ave: Nela Ave to Drennen St: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1854 | 451.2 | 87.0 | 24.0 | 0.1030 | 232.37 | 190.96 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1576 | 547.2 | 155.4 | 19.8 | 0.1060 | 239.55 | 167.06 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1514 | 313.8 | 29.4 | 34.2 | 0.1050 | 131.97 | 158.97 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1963 | 363.6 | 39.0 | 29.5 | 0.1060 | 198.26 | 208.08 |

*Traffic Volumes are obtained from the latest FDOT Counts

Orange Ave: Nela Ave to Drennen St: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1854 | 315.7 | 49.2 | 34.3 | 0.1020 | 162.59 | 189.11 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1576 | 281.6 | 22.8 | 38.5 | 0.1050 | 123.28 | 165.48 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1514 | 286.2 | 34.2 | 37.5 | 0.1030 | 120.36 | 155.94 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1963 | 318.6 | 26.4 | 33.7 | 0.1060 | 173.73 | 208.08 |

*Traffic Volumes are obtained from the latest FDOT Counts

Orange Ave: Nela Ave to Drennen St: Before \& After Study Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 364.34 | 282.95 | 437.82 | 297.00 |
| Total Fuel Consumption (gallons) | 349.93 | 345.05 | 375.13 | 373.56 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 1,390.04$ | $\$ 2,384.41$ |
| Annual User Benefit | $\$ 417,010.96$ | $\$ 715,322.12$ |
| Total Annual User Benefit $=$ | $\$ 1,132,333.09$ |  |
| Total Signal Retiming Annual Cost | $\$ 16,309.01$ |  |
| User Benefit / Cost Ratio | 69.43 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Semoran Blvd (SR 436)

## Aloma Ave to Baldwin Park St

## TABLE 7

Year 2010 METROPLAN Orlando Travel Time Study SR 436 - Northbound Direction Summary - Before Condition


## TABLE 7

Year 2010 METROPLAN Orlando Travel Time Study SR 436 - Southbound Direction Summary - Before Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 475 | 8 | Signal | 49.8 | 36.6 | 1 | 6.5 | F | 0.13 |  |
| SR 426 to University Blvd | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 8 | Signal | 49.8 | 2.4 | 1 | 35.4 | B | 0.71 |  |
| University Blvd to Banchory Rd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 8 | Signal | 55.2 | 5.4 | 1 | 32.6 | C | 0.65 |  |
| Banchory Rd to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 0 | 50 | 4,066 | 8 | Signal | 64.2 | 1.2 | 1 | 43.2 | A | 0.86 |  |
| Hanging Moss Rd to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 8 | Signal | 30.6 | 4.8 | 1 | 32.9 | C | 0.66 |  |
| TOTAL |  |  |  |  |  |  | 50 | 11,246 |  |  | 249.6 | 50.4 | 1 | 30.7 | c | 0.61 | 0.074 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 475 | 6 | Signal | 19.8 | 7.2 | 1 | 16.4 | E | 0.33 |  |
| SR 426 to University Blvd | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 6 | Signal | 64.2 | 6.0 | 1 | 27.5 | C | 0.55 |  |
| University Blvd to Banchory Rd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 6 | Signal | 72.6 | 15.0 | 1 | 24.8 | D | 0.50 |  |
| Banchory Rd to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 0 | 50 | 4,066 | 6 | Signal | 78.0 | 8.4 | 1 | 35.5 | B | 0.71 |  |
| Hanging Moss Rd to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 6 | Signal | 36.0 | 9.6 | 1 | 28.0 | c | 0.56 |  |
| total |  |  |  |  |  |  | 50 | 11,246 |  |  | 270.6 | 46.2 | 1 | 28.3 | c | 0.57 | 0.075 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type defi | ained from the | test Orlan | Urban Area Transportation | Sudy (OU | ATS) Mod |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lanes | he approach | e directio | of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 7

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Northbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Type }^{1}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel Time <br> (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) Los |  |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 634 | 9 | Signal | 24.6 | 12.0 | 1 | 17.6 | E | 0.35 |  |
| Baldwin Park St to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 9 | Signal | 25.2 | 2.4 | 1 | 40.0 | B | 0.80 |  |
| Hanging Moss Rd to Banchory Rd | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 4,066 | 9 | Signal | 57.0 | 2.4 | 1 | 48.6 | A | 0.97 |  |
| Banchory Rd to University Blvd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 9 | Signal | 52.2 | 12.0 | 1 | 34.5 | B | 0.69 |  |
| University Blvd to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 9 | Signal | 55.2 | 8.4 | 1 | 32.0 | C | 0.64 |  |
| total |  |  |  |  |  |  | 50 | 11,405 |  |  | 214.2 | 37.2 | 1 | 36.3 | B | 0.73 | 0.075 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 634 | 6 | Signal | 18.0 | 4.8 | 1 | 24.0 | D | 0.48 |  |
| Baldwin Park St to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 6 | Signal | 21.6 | 0.0 | 1 | 46.7 | A | 0.93 |  |
| Hanging Moss Rd to Banchory Rd | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 4,066 | 6 | Signal | 65.4 | 2.4 | 1 | 42.4 | A | 0.85 |  |
| Banchory Rd to University Blvd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 6 | Signal | 69.0 | 28.8 | 1 | 26.1 | D | 0.52 |  |
| University Blvd to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 6 | Signal | 99.0 | 33.6 | 1 | 17.8 | E | 0.36 |  |
| total |  |  |  |  |  |  | 50 | 11,405 |  |  | 273.0 | 69.6 | 1 | 28.5 | C | 0.57 | 0.075 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type defin | ained from the | ast Orland | Urban Area Transportation | Study (OU | ATS) Mod |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lanes arear | approa | directi | avel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 7

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel <br> Time <br> (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) |  |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 475 | 9 | Signal | 42.0 | 30.0 | 1 | 7.7 | F | 0.15 |  |
| SR 426 to University Blvd | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 9 | Signal | 44.4 | 1.8 | 1 | 39.7 | B | 0.79 |  |
| University Blvd to Banchory Rd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 9 | Signal | 46.8 | 8.4 | 1 | 38.5 | B | 0.77 |  |
| Banchory Rd to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 0 | 50 | 4,066 | 9 | Signal | 58.8 | 0.0 | 1 | 47.1 | A | 0.94 |  |
| Hanging Moss Rd to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 9 | Signal | 22.8 | 0.0 | 1 | 44.2 | A | 0.88 |  |
| total |  |  |  |  |  |  | 50 | 11,246 |  |  | 214.8 | 40.2 | 1 | 35.7 | B | 0.71 | 0.073 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to SR 426 | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 475 | 6 | Signal | 15.0 | 6.0 | 1 | 21.6 | D | 0.43 |  |
| SR 426 to University Blvd | Orange | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,587 | 6 | Signal | 53.4 | 7.2 | 1 | 33.0 | C | 0.66 |  |
| University Blvd to Banchory Rd | Orange | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,640 | 6 | Signal | 63.6 | 14.4 | 1 | 28.3 | C | 0.57 |  |
| Banchory Rd to Hanging Moss Rd | Orange | Arterial | Residential Area | 1 | 3 | 0 | 50 | 4,066 | 6 | Signal | 60.0 | 3.0 | 1 | 46.2 | A | 0.92 |  |
| Hanging Moss Rd to Baldwin Park St | Orange | Arterial | Residential Area | 1 | 3 | 1 | 50 | 1,478 | 6 | Signal | 26.4 | 0.0 | 1 | 38.2 | B | 0.76 |  |
| TOTAL |  |  |  |  |  |  | 50 | 11,246 |  |  | 218.4 | 30.6 | 1 | 35.1 | B | 0.70 | 0.074 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type defin | ained from the | test Orlan | Urban Area Transportation | Study (OU | ATS) Mod |  |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Thror |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



Level of Services:


Travel Time Study


$0.5 \quad 1_{1}^{\text {Miles }}$

SR 436: Aloma Ave to Baldwin Park St: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1992 | 253.8 | 52.2 | 30.6 | 0.0750 | 140.44 | 149.40 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2762 | 329.4 | 97.2 | 23.6 | 0.0780 | 252.72 | 215.44 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2192 | 249.6 | 50.4 | 30.7 | 0.0740 | 151.98 | 162.21 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2124 | 270.6 | 46.2 | 28.3 | 0.0750 | 159.65 | 159.30 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 436: Aloma Ave to Baldwin Park St: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1992 | 214.2 | 37.2 | 36.3 | 0.0750 | 118.52 | 149.40 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2762 | 273.0 | 69.6 | 28.5 | 0.0750 | 209.45 | 207.15 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2192 | 214.8 | 40.2 | 35.7 | 0.0730 | 130.79 | 160.02 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2124 | 218.4 | 30.6 | 35.1 | 0.0740 | 128.86 | 157.18 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 436: Aloma Ave to Baldwin Park St: Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 292.41 | 249.31 | 412.38 | 338.31 |
| Total Fuel Consumption (gallons) | 311.61 | 309.42 | 374.74 | 364.33 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 734.94$ | $\$ 1,282.79$ |
| Annual User Benefit | $\$ 220,483.41$ | $\$ 384,838.06$ |
| Total Annual User Benefit $=$ | $\$ 605,321.47$ |  |
| Total Signal Retiming Annual Cost | $\$ 8,345.03$ |  |
| User Benefit / Cost Ratio | 72.54 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Curry Ford Rd (SR 552)

## Conway Rd to Woodgate Blvd

## TABLE 8

Year 2010 METROPLAN Orlando Travel Time Study
Curry Ford Road - Eastbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | RightTurnLanes $^{2}$ | $\begin{aligned} & \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | $\begin{aligned} & \text { Roadway } \\ & \text { Class } \end{aligned}$ | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 422 | 6 | Signal | 52.2 | 36.6 | II | 5.5 | F | 0.16 |  |
| Conway Rd to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 6 | Signal | 39.6 | 1.2 | II | 34.5 | B | 0.99 |  |
| Gaston Foster Rd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 6 | Signal | 66.6 | 7.2 | 11 | 33.5 | B | 0.84 |  |
| Dixie Belle Dr/Bahia Ave to Semoran Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 2 | 1 | 40 | 1,373 | 6 | Signal | 46.8 | 18.0 | II | 20.0 | D | 0.50 |  |
| Semoran Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,851 | 6 | Signal | 60.0 | 4.2 | II | 32.4 | B | 0.72 |  |
| Oxalis Ave to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,954 | 6 | Signal | 52.8 | 13.2 | II | 25.2 | c | 0.56 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,880 |  |  | 318.0 | 80.4 | II | 25.5 | C | 0.57 | 0.079 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 422 | 5 | Signal | 78.0 | 54.0 | II | 3.7 | F | 0.11 |  |
| Conway Rd to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 5 | Signal | 40.2 | 0.0 | II | 34.0 | B | 0.97 |  |
| Gaston Foster Rd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 5 | Signal | 61.8 | 0.0 | II | 36.1 | A | 0.90 |  |
| Dixie Belle Dr/Bahia Ave to Semoran Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 2 | 1 | 40 | 1,373 | 5 | Signal | 147.0 | 104.4 | 11 | 6.4 | F | 0.16 |  |
| Semoran Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,851 | 5 | Signal | 99.6 | 42.0 | II | 19.5 | D | 0.43 |  |
| Oxalis Ave to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,954 | 5 | Signal | 57.6 | 9.6 | II | 23.1 | c | 0.51 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,880 |  |  | 484.2 | 210.0 | II | 16.7 | E | 0.37 | $0.082 \mathrm{gal} / \mathrm{veh}$ |

## Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

TABLE 8
Year 2010 METROPLAN Orlando Travel Time Study
Curry Ford Road - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 845 | 6 | Signal | 19.8 | 3.0 | II | 29.1 | B | 0.65 |  |
| Woodgate Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,901 | 6 | Signal | 45.6 | 11.4 | II | 28.4 | B | 0.63 |  |
| Oxalis Ave to Semoran Blvd | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 40 | 2,851 | 6 | Signal | 130.2 | 63.6 | II | 14.9 | E | 0.37 |  |
| Semoran Blvd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 1,373 | 6 | Signal | 111.0 | 78.0 | II | 8.4 | F | 0.21 |  |
| Dixie Belle Dr/Bahia Ave to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 6 | Signal | 58.2 | 0.0 | II | 38.3 | A | 0.96 |  |
| Gaston Foster Rd to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 6 | Signal | 55.2 | 6.0 | II | 24.8 | C | 0.71 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,250 |  |  | 420.0 | 162.0 | II | 19.9 | D | 0.50 | $0.084 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 845 | 4 | Signal | 19.8 | 0.0 | 11 | 29.1 | B | 0.65 |  |
| Woodgate Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,901 | 4 | Signal | 29.4 | 0.6 | II | 44.1 | A | 0.98 |  |
| Oxalis Ave to Semoran Blvd | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 40 | 2,851 | 4 | Signal | 72.6 | 19.8 | II | 26.8 | C | 0.67 |  |
| Semoran Blvd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 1,373 | 4 | Signal | 89.4 | 58.8 | II | 10.5 | F | 0.26 |  |
| Dixie Belle Dr/Bahia Ave to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 4 | Signal | 57.0 | 0.0 | II | 39.2 | A | 0.98 |  |
| Gaston Foster Rd to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 4 | Signal | 109.8 | 53.4 | II | 12.5 | F | 0.36 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,250 |  |  | 378.0 | 132.6 | II | 22.1 | c | 0.55 | $0.084 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions $w$ | tained from the | est Orlan | do Urban Area Transportatio | Study | UATS) | del. |  |  |  |  |  |  |  |  |  |  |  |
| 2. The Through lanes and Turn lanes are provi | or the approach | the directio | on of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 8

Year 2010 METROPLAN Orlando Travel Time Study
Curry Ford Road - Eastbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | $\begin{gathered} \text { Facility }^{\text {Type }^{1}} \\ \hline \end{gathered}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{array}{c\|} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | RightTurnLanes $^{2}$ | $\begin{aligned} & \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | $\begin{aligned} & \text { Roadway } \\ & \text { Class } \end{aligned}$ | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 422 | 8 | Signal | 8.4 | 0.0 | II | 34.3 | B | 0.98 |  |
| Conway Rd to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 8 | Signal | 37.2 | 4.8 | II | 36.8 | A | 1.05 |  |
| Gaston Foster Rd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 8 | Signal | 64.2 | 9.0 | 11 | 34.8 | B | 0.87 |  |
| Dixie Belle Dr/Bahia Ave to Semoran Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 2 | 1 | 40 | 1,373 | 8 | Signal | 39.6 | 13.8 | 11 | 23.6 | c | 0.59 |  |
| Semoran Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,851 | 8 | Signal | 61.2 | 12.0 | II | 31.8 | B | 0.71 |  |
| Oxalis Ave to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,954 | 8 | Signal | 31.2 | 0.6 | II | 42.7 | A | 0.95 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,880 |  |  | 241.8 | 40.2 | II | 33.5 | B | 0.74 | 0.078 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 422 | 5 | Signal | 12.6 | 1.8 | 11 | 22.9 | c | 0.65 |  |
| Conway Rd to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 5 | Signal | 36.0 | 0.0 | II | 38.0 | A | 1.09 |  |
| Gaston Foster Rd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 5 | Signal | 80.4 | 19.8 | II | 27.8 | C | 0.69 |  |
| Dixie Belle Dr/Bahia Ave to Semoran Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 2 | 1 | 40 | 1,373 | 5 | Signal | 97.8 | 58.2 | 11 | 9.6 | F | 0.24 |  |
| Semoran Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,851 | 5 | Signal | 62.4 | 11.4 | II | 31.2 | B | 0.69 |  |
| Oxalis Ave to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,954 | 5 | Signal | 36.0 | 1.2 | II | 37.0 | A | 0.82 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,880 |  |  | 325.2 | 92.4 | II | 24.9 | C | 0.55 | $0.080 \mathrm{gal} / \mathrm{veh}$ |

## Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

TABLE 8
Year 2010 METROPLAN Orlando Travel Time Study
Curry Ford Road - Westbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 845 | 7 | Signal | 12.0 | 0.0 | 11 | 48.0 | A | 1.07 |  |
| Woodgate Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,901 | 7 | Signal | 25.8 | 0.6 | 11 | 50.2 | A | 1.12 |  |
| Oxalis Ave to Semoran Blvd | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 40 | 2,851 | 7 | Signal | 85.8 | 25.8 | II | 22.7 | c | 0.57 |  |
| Semoran Blvd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 1,373 | 7 | Signal | 61.8 | 31.2 | 11 | 15.1 | E | 0.38 |  |
| Dixie Belle Dr/Bahia Ave to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 7 | Signal | 57.0 | 0.0 | 11 | 39.2 | A | 0.98 |  |
| Gaston Foster Rd to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 7 | Signal | 76.8 | 27.6 | II | 17.8 | D | 0.51 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,250 |  |  | 319.2 | 85.2 | 11 | 26.2 | C | 0.65 | $0.083 \mathrm{gal/veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Woodgate Blvd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 845 | 5 | Signal | 12.6 | 0.0 | 11 | 45.7 | A | 1.02 |  |
| Woodgate Blvd to Oxalis Ave | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,901 | 5 | Signal | 24.6 | 0.0 | " | 52.7 | A | 1.17 |  |
| Oxalis Ave to Semoran Blvd | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 40 | 2,851 | 5 | Signal | 66.0 | 13.8 | II | 29.5 | в | 0.74 |  |
| Semoran Blvd to Dixie Belle Dr/Bahia Ave | City of Orlando | Arterial | Outlying Business District | 0 | 2 | 0 | 40 | 1,373 | 5 | Signal | 88.2 | 54.0 | II | 10.6 | F | 0.27 |  |
| Dixie Belle Dr/Bahia Ave to Gaston Foster Rd | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 40 | 3,274 | 5 | Signal | 58.2 | 1.2 | 11 | 38.3 | A | 0.96 |  |
| Gaston Foster Rd to Conway Rd | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 2,006 | 5 | Signal | 87.6 | 34.8 | 11 | 15.6 | E | 0.45 |  |
| TOTAL |  |  |  |  |  |  | 40 | 12,250 |  |  | 337.2 | 103.8 | II | 24.8 | C | 0.62 | $0.083 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




## Curry Ford Rd: Conway Rd to Woodgate Blvd: Before \& After Study

Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 752 | 318.0 | 80.4 | 25.5 | 0.0790 | 66.43 | 59.41 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1767 | 484.2 | 210.0 | 16.7 | 0.0820 | 237.66 | 144.89 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1542 | 420.0 | 162.0 | 19.9 | 0.0840 | 179.90 | 129.53 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 999 | 378.0 | 132.6 | 22.1 | 0.0840 | 104.90 | 83.92 |

*Traffic Volumes are obtained from the latest FDOT Counts

## Curry Ford Rd: Conway Rd to Woodgate Blvd: Before \& After Study

Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 752 | 241.8 | 40.2 | 33.5 | 0.0780 | 50.51 | 58.66 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1767 | 325.2 | 92.4 | 24.9 | 0.0800 | 159.62 | 141.36 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1542 | 319.2 | 85.2 | 26.2 | 0.0830 | 136.72 | 127.99 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 999 | 337.2 | 103.8 | 24.8 | 0.0830 | 93.57 | 82.92 |

*Traffic Volumes are obtained from the latest FDOT Counts

Curry Ford Rd: Conway Rd to Woodgate Blvd: Before \& After Study
Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 246.33 | 187.23 | 342.56 | 253.19 |
| Total Fuel Consumption (gallons) | 188.94 | 186.64 | 228.81 | 224.28 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 1,005.51$ | $\$ 1,523.77$ |
| Annual User Benefit | $\$ 301,654.04$ | $\$ 457,130.52$ |
| Total Annual User Benefit $=$ | $\$ 758,784.55$ |  |
| Total Signal Retiming Annual Cost | $\$ 10,402.71$ |  |
| User Benefit / Cost Ratio | 72.94 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Hoffner Ave/Narcoossee Rd (SR 15) <br> Goldenrod Rd to Lee Vista Blvd

## TAbLE 9

Year 2010 METROPLAN Orlando Travel Time Study
Hoffner Road/ Narcoossee Road - Eastbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Aver | peed | Avg Speed 1 | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to New Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 317 | 6 | Signal | 45.0 | 34.8 | II | 4.8 | F | 0.11 |  |
| New Goldenrod Rd to Old Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 45 | 1,162 | 6 | Signal | 27.0 | 5.4 | 11 | 29.3 | B | 0.65 |  |
| Old Goldenrod Rd to Lee Vista Blvd | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 45 | 5,069 | 6 | Signal | 102.6 | 12.0 | II | 33.7 | B | 0.75 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,547 |  |  | 174.6 | 52.2 | II | 25.6 | C | 0.57 | $0.045 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to New Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 317 | 8 | Signal | 47.4 | 37.2 | II | 4.6 | F | 0.10 |  |
| New Goldenrod Rd to Old Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 45 | 1,162 | 8 | Signal | 35.4 | 7.8 | 11 | 22.4 | C | 0.50 |  |
| Old Goldenrod Rd to Lee Vista Blvd | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 45 | 5,069 | 8 | Signal | 152.4 | 48.6 | II | 22.7 | c | 0.50 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,547 |  |  | 235.2 | 93.6 | II | 19.0 | D | 0.42 | $0.046 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## table 9

Year 2010 METROPLAN Orlando Travel Time Study
Hoffner Road/ Narcoossee Road - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | Left Turn Lanes ${ }^{2}$ | Thru Lanes ${ }^{2}$ | Right <br> Turn <br> Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Lee Vista Blvd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 264 | 6 | Signal | 23.4 | 16.2 | II | 7.7 | F | 0.17 |  |
| Lee Vista Blvd to Old Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 5,069 | 6 | Signal | 95.4 | 1.2 | II | 36.2 | A | 0.81 |  |
| Old Goldenrod Rd to New Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 1,162 | 6 | Signal | 55.2 | 25.8 | II | 14.3 | E | 0.32 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,494 |  |  | 174.0 | 43.2 | II | 25.4 | C | 0.57 | $0.046 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Lee Vista Blvd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 264 | 8 | Signal | 25.2 | 19.2 | II | 7.1 | F | 0.16 |  |
| Lee Vista Blvd to Old Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 5,069 | 8 | Signal | 94.8 | 2.4 | 11 | 36.5 | A | 0.81 |  |
| Old Goldenrod Rd to New Goldenrod Rd | City of Orlando | Arterial | Residential Area | 1 | 1 | 1 | 45 | 1,162 | 8 | Signal | 49.8 | 20.4 | II | 15.9 | E | 0.35 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,494 |  |  | 169.8 | 42.0 | 11 | 26.1 | c | 0.58 | $0.046 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

Semoran Blvd (SR 436)

## Dahlia Dr to T.G. Lee Blvd

SR 436 - Northbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Tin | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | ( sec ) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 845 | 6 | Signal | 58.2 | 34.8 | 1 | 9.9 | F | 0.22 |  |
| T.G. Lee Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 6 | Signal | 23.4 | 3.6 | 1 | 30.8 | C | 0.68 |  |
| Hazeltine National Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,059 | 6 | Signal | 36.6 | 2.4 | 1 | 38.4 | B | 0.77 |  |
| Lee Vista Blvd to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,112 | 6 | Signal | 35.4 | 4.8 | 1 | 40.7 | B | 0.81 |  |
| Bent Pine Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 6 | Signal | 45.6 | 13.2 | 1 | 31.6 | C | 0.63 |  |
| Hoffner Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,584 | 6 | Signal | 39.6 | 8.4 | 1 | 27.3 | C | 0.55 |  |
| Turnbull Dr to Gatin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 6 | Signal | 57.0 | 3.0 | 1 | 44.2 | A | 0.88 |  |
| Gattin Ave to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 6 | Signal | 57.6 | 30.6 | 1 | 15.6 | F | 0.31 |  |
| Pershing Ave to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 6 | Signal | 38.4 | 0.0 | 1 | 44.1 | A | 0.88 |  |
| Lake Margaret Dr to Michigan St | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 3,168 | 6 | Signal | 45.0 | 0.0 | 1 | 48.0 | A | 0.96 |  |
| Michigan St to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 6 | Signal | 13.2 | 0.0 | 1 | 46.4 | A | 1.03 |  |
| Grant St to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,323 | 6 | Signal | 52.8 | 7.2 | 1 | 30.0 | C | 0.67 |  |
| Curry Ford Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,478 | 6 | Signal | 25.8 | 0.0 | 1 | 39.1 | B | 0.87 |  |
| La Costa Dr to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 2,006 | 6 | Signal | 48.6 | 13.2 | 1 | 28.1 | C | 0.63 |  |
| Stonewall Jackson Rd to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 6 | Signal | 78.0 | 28.8 | 1 | 18.5 | E | 0.41 |  |
| Lake Underhill Rd to Yew Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 634 | 6 | Signal | 10.2 | 6.0 | 1 | 42.4 | A | 0.94 |  |
| Yew Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 6 | Signal | 19.2 | 2.4 | 1 | 31.9 | C | 0.71 |  |
| Kalmia Dr to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 950 | 6 | Signal | 31.8 | 13.2 | 1 | 20.4 | E | 0.45 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,733 |  |  | 716.4 | 171.6 | 1 | 30.2 | C | 0.60 | 0.208 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 845 | 6 | Signal | 75.6 | 52.8 | 1 | 7.6 | F | 0.17 |  |
| T.G. Lee Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 6 | Signal | 43.8 | 15.0 | 1 | 16.4 | E | 0.37 |  |
| Hazeltine National Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,059 | 6 | Signal | 36.6 | 0.0 | 1 | 38.4 | B | 0.77 |  |
| Lee Vista Blvd to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,112 | 6 | Signal | 29.4 | 0.0 | 1 | 49.0 | A | 0.98 |  |
| Bent Pine Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 6 | Signal | 48.0 | 16.2 | 1 | 30.0 | C | 0.60 |  |
| Hoffner Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,584 | 6 | Signal | 24.0 | 0.0 | 1 | 45.0 | A | 0.90 |  |
| Turnbull Dr to Gatiin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 6 | Signal | 51.6 | 0.0 | 1 | 48.8 | A | 0.98 |  |
| Gatin Ave to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 6 | Signal | 96.6 | 67.2 | 1 | 9.3 | F | 0.19 |  |
| Pershing Ave to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 6 | Signal | 37.8 | 0.0 | 1 | 44.8 | A | 0.90 |  |
| Lake Margaret Dr to Michigan St | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 3,168 | 6 | Signal | 49.8 | 3.0 | 1 | 43.4 | A | 0.87 |  |
| Michigan St to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 6 | Signal | 13.8 | 0.0 | 1 | 44.3 | A | 0.99 |  |
| Grant St to Cury Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,323 | 6 | Signal | 123.6 | 76.8 | 1 | 12.8 | F | 0.28 |  |
| Curry Ford Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,478 | 6 | Signal | 49.2 | 21.6 | 1 | 20.5 | E | 0.46 |  |
| La Costa Dr to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 2,006 | 6 | Signal | 42.0 | 9.0 | 1 | 32.6 | C | 0.72 |  |
| Stonewall Jackson Rd to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 6 | Signal | 65.4 | 22.2 | 1 | 22.0 | D | 0.49 |  |
| Lake Underhill Rd to Yew Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 634 | 6 | Signal | 10.8 | 0.0 | 1 | 40.0 | B | 0.89 |  |
| Yew Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 6 | Signal | 13.8 | 0.0 | 1 | 44.3 | A | 0.99 |  |
| Kalmia Dr to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 950 | 6 | Signal | 16.8 | 1.8 | 1 | 38.6 | B | 0.86 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,733 |  |  | 828.6 | 285.6 | 1 | 26.1 | D | 0.52 | 0.207 gal/veh |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

TABLE 10
Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Southbound Direction Summary - Before Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{array}{c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \\ \hline \end{gathered}$ | Distance <br> (ft) | \# Runs | $\begin{array}{l\|} \hline \text { Traffic } \\ \text { Control } \\ \text { Device } \\ \hline \end{array}$ | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 422 | 6 | Signal | 18.0 | 6.6 | 1 | 16.0 | F | 0.36 |  |
| Dahlia Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 950 | 6 | Signal | 17.4 | 0.0 | 1 | 37.2 | B | 0.83 |  |
| Kalmia Dr to Yew Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 898 | 6 | Signal | 44.4 | 23.4 | 1 | 13.8 | F | 0.31 |  |
| Yew Dr to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 634 | 6 | Signal | 26.4 | 0.0 | 1 | 16.4 | E | 0.36 |  |
| Lake Underhill Rd to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 6 | Signal | 43.8 | 1.8 | 1 | 32.9 | C | 0.73 |  |
| Stonewall Jackson Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,006 | 6 | Signal | 32.4 | 0.0 | 1 | 42.2 | A | 0.94 |  |
| La Costa Dr to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,478 | 6 | Signal | 42.6 | 16.2 | 1 | 23.7 | D | 0.53 |  |
| Curry Ford Rd to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,323 | 6 | Signal | 36.6 | 0.0 | 1 | 43.3 | A | 0.96 |  |
| Grant St to Michigan St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 898 | 6 | Signal | 13.8 | 0.0 | 1 | 44.3 | A | 0.89 |  |
| Michigan St to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,168 | 6 | Signal | 85.2 | 28.2 | 1 | 25.4 | D | 0.51 |  |
| Lake Margaret Dr to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 6 | Signal | 47.4 | 6.6 | 1 | 35.7 | B | 0.71 |  |
| Pershing Ave to Gatin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 6 | Signal | 18.6 | 0.0 | 1 | 48.4 | A | 0.97 |  |
| Gatin Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 6 | Signal | 71.4 | 13.2 | 1 | 35.3 | B | 0.71 |  |
| Turnbull Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,584 | 6 | Signal | 95.4 | 60.0 | 1 | 11.3 | F | 0.23 |  |
| Hoffner Ave to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 50 | 2,112 | 6 | Signal | 49.8 | 7.8 | 1 | 28.9 | c | 0.58 |  |
| Bent Pine Dr to Lee Vista Blva | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 6 | Signal | 42.0 | 2.4 | 1 | 34.3 | B | 0.69 |  |
| Lee Vista Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,059 | 6 | Signal | 33.0 | 0.0 | 1 | 42.5 | A | 0.95 |  |
| Hazeltine National Dr to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 6 | Signal | 15.0 | 0.0 | 1 | 48.0 | A | 1.07 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,310 |  |  | 733.2 | 166.2 | 1 | 29.1 | C | 0.58 | $0.206 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 422 | 6 | Signal | 6.6 | 0.0 | 1 | 43.6 | A | 0.97 |  |
| Dahlia Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 950 | 6 | Signal | 30.0 | 8.4 | 1 | 21.6 | D | 0.48 |  |
| Kalmia Dr to Yew Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 898 | 6 | Signal | 40.8 | 13.8 | 1 | 15.0 | F | 0.33 |  |
| Yew Dr to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 634 | 6 | Signal | 29.4 | 9.6 | 1 | 14.7 | F | 0.33 |  |
| Lake Underhill Rd to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 6 | Signal | 52.8 | 10.8 | 1 | 27.3 | c | 0.61 |  |
| Stonewall Jackson Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,006 | 6 | Signal | 36.0 | 0.0 | 1 | 38.0 | B | 0.84 |  |
| La Costa Dr to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,478 | 6 | Signal | 103.2 | 62.4 | 1 | 9.8 | F | 0.22 |  |
| Curry Ford Rd to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,323 | 6 | Signal | 39.0 | 0.0 | 1 | 40.6 | B | 0.90 |  |
| Grant St to Michigan St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 898 | 6 | Signal | 15.6 | 0.0 | 1 | 39.2 | B | 0.78 |  |
| Michigan St to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,168 | 6 | Signal | 60.0 | 7.2 | 1 | 36.0 | B | 0.72 |  |
| Lake Margaret Dr to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 6 | Signal | 52.8 | 7.8 | 1 | 32.0 | c | 0.64 |  |
| Pershing Ave to Gatin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 6 | Signal | 20.4 | 0.0 | 1 | 44.1 | A | 0.88 |  |
| Gatin Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 6 | Signal | 56.4 | 0.0 | 1 | 44.7 | A | 0.89 |  |
| Turnbull Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,584 | 6 | Signal | 44.4 | 16.8 | 1 | 24.3 | D | 0.49 |  |
| Hoffner Ave to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 50 | 2,112 | 6 | Signal | 31.8 | 0.0 | 1 | 45.3 | A | 0.91 |  |
| Bent Pine Dr to Lee Vista Blva | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 6 | Signal | 47.4 | 10.8 | 1 | 30.4 | c | 0.61 |  |
| Lee Vista Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,059 | 6 | Signal | 45.0 | 10.8 | 1 | 31.2 | C | 0.69 |  |
| Hazeltine National Dr to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 6 | Signal | 56.4 | 36.6 | 1 | 12.8 | F | 0.28 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,310 |  |  | 768.0 | 195.0 | 1 | 27.8 | c | 0.56 | $0.206 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

TABLE 10
Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Northbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadway | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Tim | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | ( sec ) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 845 | 4 | Signal | 34.8 | 17.4 | 1 | 16.6 | E | 0.37 |  |
| T.G. Lee Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 4 | Signal | 16.8 | 0.0 | 1 | 42.9 | A | 0.95 |  |
| Hazeltine National Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,059 | 4 | Signal | 30.0 | 0.0 | 1 | 46.8 | A | 0.94 |  |
| Lee Vista Blvd to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,112 | 4 | Signal | 29.0 | 0.0 | 1 | 49.7 | A | 0.99 |  |
| Bent Pine Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 4 | Signal | 34.2 | 3.0 | 1 | 42.1 | A | 0.84 |  |
| Hoffner Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,584 | 4 | Signal | 22.0 | 0.0 | 1 | 49.1 | A | 0.98 |  |
| Turnbull Dr to Gatin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 4 | Signal | 51.0 | 0.0 | 1 | 49.4 | A | 0.99 |  |
| Gattin Ave to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 18.0 | 0.0 | 1 | 50.0 | A | 1.00 |  |
| Pershing Ave to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 4 | Signal | 35.4 | 0.0 | 1 | 47.8 | A | 0.96 |  |
| Lake Margaret Dr to Michigan St | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 3,168 | 4 | Signal | 42.6 | 0.0 | 1 | 50.7 | A | 1.01 |  |
| Michigan St to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 4 | Signal | 12.6 | 0.0 | 1 | 48.6 | A | 1.08 |  |
| Grant St to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,323 | 4 | Signal | 120.0 | 72.0 | 1 | 13.2 | F | 0.29 |  |
| Curry Ford Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,478 | 4 | Signal | 25.8 | 0.0 | 1 | 39.1 | B | 0.87 |  |
| La Costa Dr to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 2,006 | 4 | Signal | 35.4 | 1.2 | 1 | 38.6 | в | 0.86 |  |
| Stonewall Jackson Rd to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 45.0 | 3.6 | 1 | 32.0 | C | 0.71 |  |
| Lake Underhill Rd to Yew Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 634 | 4 | Signal | 10.8 | 0.0 | 1 | 40.0 | B | 0.89 |  |
| Yew Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 4 | Signal | 12.6 | 0.0 | 1 | 48.6 | A | 1.08 |  |
| Kalmia Dr to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 950 | 4 | Signal | 47.4 | 31.8 | 1 | 13.7 | F | 0.30 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,733 |  |  | 623.4 | 129.0 | 1 | 34.7 | B | 0.69 | $0.206 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 845 | 4 | Signal | 28.8 | 10.8 | 1 | 20.0 | E | 0.44 |  |
| T.G. Lee Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 4 | Signal | 16.8 | 0.0 | 1 | 42.9 | A | 0.95 |  |
| Hazeltine National Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,059 | 4 | Signal | 34.8 | 1.2 | 1 | 40.3 | B | 0.81 |  |
| Lee Vista Blvd to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,112 | 4 | Signal | 34.8 | 0.0 | 1 | 41.4 | B | 0.83 |  |
| Bent Pine Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 4 | Signal | 33.0 | 0.0 | 1 | 43.6 | A | 0.87 |  |
| Hoffner Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,584 | 4 | Signal | 21.6 | 0.0 | 1 | 50.0 | A | 1.00 |  |
| Turnbull Dr to Gatin Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 4 | Signal | 51.0 | 0.0 | 1 | 49.4 | A | 0.99 |  |
| Gatio Ave to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 19.8 | 0.0 | 1 | 45.5 | A | 0.91 |  |
| Pershing Ave to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 4 | Signal | 34.2 | 0.0 | 1 | 49.5 | A | 0.99 |  |
| Lake Margaret Dr to Michigan St | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 3,168 | 4 | Signal | 48.0 | 0.0 | 1 | 45.0 | A | 0.90 |  |
| Michigan St to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 4 | Signal | 17.4 | 0.0 | 1 | 35.2 | B | 0.78 |  |
| Grant St to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,323 | 4 | Signal | 120.0 | 67.2 | 1 | 13.2 | F | 0.29 |  |
| Curry Ford Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,478 | 4 | Signal | 26.4 | 0.0 | 1 | 38.2 | B | 0.85 |  |
| La Costa Dr to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 2,006 | 4 | Signal | 44.4 | 12.6 | 1 | 30.8 | C | 0.68 |  |
| Stonewall Jackson Rd to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 46.2 | 9.6 | 1 | 31.2 | C | 0.69 |  |
| Lake Underhill Rd to Yew Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 634 | 4 | Signal | 9.6 | 0.0 | 1 | 45.0 | A | 1.00 |  |
| Yew Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 4 | Signal | 13.0 | 0.0 | 1 | 47.1 | A | 1.05 |  |
| Kalmia Dr to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 950 | 4 | Signal | 13.0 | 0.0 | 1 | 49.8 | A | 1.11 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,733 |  |  | 612.8 | 101.4 | 1 | 35.3 | B | 0.71 | $0.205 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

TABLE 10
Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Southbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadway | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility |  | Turn |  | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 422 | 4 | Signal | 32.4 | 19.2 | 1 | 8.9 | F | 0.20 |  |
| Dahlia Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 950 | 4 | Signal | 19.2 | 0.0 | 1 | 33.7 | C | 0.75 |  |
| Kalmia Dr to Yew Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 898 | 4 | Signal | 15.0 | 0.0 | 1 | 40.8 | B | 0.91 |  |
| Yew Dr to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 634 | 4 | Signal | 28.8 | 0.0 | 1 | 15.0 | F | 0.33 |  |
| Lake Underhill Rd to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 39.6 | 2.4 | 1 | 36.4 | B | 0.81 |  |
| Stonewall Jackson Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,006 | 4 | Signal | 40.8 | 7.2 | 1 | 33.5 | C | 0.75 |  |
| La Costa Dr to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,478 | 4 | Signal | 27.0 | 0.0 | 1 | 37.3 | B | 0.83 |  |
| Curry Ford Rd to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,323 | 4 | Signal | 33.6 | 0.0 | 1 | 47.1 | A | 1.05 |  |
| Grant St to Michigan St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 898 | 4 | Signal | 12.0 | 0.0 | 1 | 51.0 | A | 1.02 |  |
| Michigan St to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,168 | 4 | Signal | 42.0 | 0.0 | 1 | 51.4 | A | 1.03 |  |
| Lake Margaret Dr to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 4 | Signal | 34.2 | 0.0 | 1 | 49.5 | A | 0.99 |  |
| Pershing Ave to Gatio Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 18.0 | 0.0 | 1 | 50.0 | A | 1.00 |  |
| Gatin Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 4 | Signal | 49.5 | 0.0 | 1 | 50.9 | A | 1.02 |  |
| Turnbull Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,584 | 4 | Signal | 22.0 | 0.0 | 1 | 49.1 | A | 0.98 |  |
| Hoffner Ave to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 50 | 2,112 | 4 | Signal | 34.2 | 0.0 | 1 | 42.1 | A | 0.84 |  |
| Bent Pine Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 4 | Signal | 30.0 | 0.0 | 1 | 48.0 | A | 0.96 |  |
| Lee Vista Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,059 | 4 | Signal | 29.0 | 0.0 | 1 | 48.4 | A | 1.08 |  |
| Hazeltine National Dr to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 4 | Signal | 15.0 | 0.0 | 1 | 48.0 | A | 1.07 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,310 |  |  | 522.3 | 28.8 | 1 | 40.9 | B | 0.82 | 0.202 galveh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dahlia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 422 | 4 | Signal | 31.8 | 21.6 | 1 | 9.1 | F | 0.20 |  |
| Dahlia Dr to Kalmia Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 950 | 4 | Signal | 15.0 | 0.0 | 1 | 43.2 | A | 0.96 |  |
| Kalmia Dr to Yew Dr | City of Orlando | Arterial | Outlying Business District | 0 | 3 | 0 | 45 | 898 | 4 | Signal | 12.6 | 0.0 | 1 | 48.6 | A | 1.08 |  |
| Yew Dr to Lake Underhill Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 45 | 634 | 4 | Signal | 25.2 | 7.8 | 1 | 17.1 | E | 0.38 |  |
| Lake Underhill Rd to Stonewall Jackson Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 64.2 | 22.8 | 1 | 22.4 | D | 0.50 |  |
| Stonewall Jackson Rd to La Costa Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,006 | 4 | Signal | 37.8 | 1.8 | 1 | 36.2 | B | 0.80 |  |
| La Costa Dr to Curry Ford Rd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,478 | 4 | Signal | 136.8 | 99.0 | 1 | 7.4 | F | 0.16 |  |
| Curry Ford Rd to Grant St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,323 | 4 | Signal | 39.0 | 1.8 | 1 | 40.6 | B | 0.90 |  |
| Grant St to Michigan St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 898 | 4 | Signal | 21.0 | 4.2 | 1 | 29.1 | C | 0.58 |  |
| Michigan St to Lake Margaret Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,168 | 4 | Signal | 57.0 | 7.8 | 1 | 37.9 | B | 0.76 |  |
| Lake Margaret Dr to Pershing Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,482 | 4 | Signal | 37.8 | 0.0 | 1 | 44.8 | A | 0.90 |  |
| Pershing Ave to Gatio Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 21.0 | 6.0 | 1 | 42.9 | A | 0.86 |  |
| Gatin Ave to Turnbull Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,696 | 4 | Signal | 58.2 | 8.4 | 1 | 43.3 | A | 0.87 |  |
| Turnbull Dr to Hoffner Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,584 | 4 | Signal | 26.4 | 0.0 | 1 | 40.9 | B | 0.82 |  |
| Hoffner Ave to Bent Pine Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 50 | 2,112 | 4 | Signal | 28.8 | 0.0 | 1 | 50.0 | A | 1.00 |  |
| Bent Pine Dr to Lee Vista Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,112 | 4 | Signal | 35.4 | 0.0 | 1 | 40.7 | B | 0.81 |  |
| Lee Vista Blvd to Hazeltine National Dr | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 2,059 | 4 | Signal | 31.2 | 0.0 | 1 | 45.0 | A | 1.00 |  |
| Hazeltine National Dr to T.G. Lee Blvd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,056 | 4 | Signal | 16.0 | 0.0 | 1 | 45.0 | A | 1.00 |  |
| TOTAL |  |  |  |  |  |  | 50 | 31,310 |  |  | 695.2 | 181.2 | 1 | 30.7 | C | 0.61 | $0.205 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.



Travel Time Study


Level of Services:
Travel Time Study


SR 436: Dahlia Dr to T.G.Lee Blvd: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1758 | 716.4 | 171.6 | 30.2 | 0.2080 | 349.84 | 365.66 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2170 | 828.6 | 285.6 | 26.1 | 0.2070 | 499.46 | 449.19 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1665 | 733.2 | 166.2 | 29.1 | 0.2060 | 339.11 | 342.99 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2054 | 768.0 | 195.0 | 27.8 | 0.2060 | 438.19 | 423.12 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 436: Dahlia Dr to T.G.Lee Blvd: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1758 | 623.4 | 129.0 | 34.7 | 0.2060 | 304.43 | 362.15 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2170 | 612.8 | 101.4 | 35.3 | 0.2050 | 369.38 | 444.85 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1665 | 522.3 | 28.8 | 40.9 | 0.2020 | 241.56 | 336.33 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2054 | 695.2 | 181.2 | 30.7 | 0.2050 | 396.65 | 421.07 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 436: Dahlia Dr to T.G.Lee Blvd: Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 688.95 | 545.99 | 937.65 | 766.03 |
| Total Fuel Consumption (gallons) | 708.65 | 698.48 | 872.31 | 865.92 |


|  | BENEFITS | AM PEAK HOUR |
| :---: | :---: | :---: |
| PM PEAK HOUR |  |  |
| User Benefit Per Day | $\$ 2,446.29$ | $\$ 2,919.36$ |
| Annual User Benefit | $\$ 733,885.53$ | $\$ 875,808.79$ |
| Total Annual User Benefit $=$ | $\$ 1,609,694.32$ |  |
| Total Signal Retiming Annual Cost | $\$ 29,036.14$ |  |
| User Benefit / Cost Ratio | 55.44 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

Colonial Dr (SR 50)

## Mills Ave to Old Cheney Hwy

## TABLE 11

Year 2010 METROPLAN Orlando Travel Time Study
SR 50 - Eastbound Direction Summary - After Condition


## TABLE 11

Year 2010 METROPLAN Orlando Travel Time Study
SR 50 - Westbound Direction Summary - After Condition

|  |  |  |  | Left |  |  | Speed |  |  | Traffic | Travel | Stop |  | Roadwa | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speedl | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | ( sec ) | ( sec ) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 7 | Signal | 27.0 | 0.0 | II | 22.7 | C | 0.50 |  |
| Old Cheney Hwy to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 7 | Signal | 34.2 | 3.0 | 11 | 42.1 | A | 0.94 |  |
| Humphries Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 7 | Signal | 28.2 | 0.0 | II | 46.0 | A | 1.02 |  |
| Bennett Rd to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 7 | Signal | 21.0 | 0.0 | II | 42.9 | A | 1.07 |  |
| Herndon Ave to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 7 | Signal | 9.6 | 0.0 | II | 45.0 | A | 1.12 |  |
| Fashion Square Mall Entrance to Maguire Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,690 | 7 | Signal | 31.8 | 0.0 | II | 36.2 | A | 0.91 |  |
| Maguire Blvd to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,003 | 7 | Signal | 21.6 | 0.0 | II | 31.7 | B | 0.79 |  |
| Primerose Dr to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 7 | Signal | 20.4 | 4.2 | II | 22.9 | C | 0.57 |  |
| Coy St to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 634 | 7 | Signal | 23.4 | 4.2 | II | 18.5 | D | 0.46 |  |
| Bumby Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 7 | Signal | 38.4 | 6.6 | II | 23.4 | C | 0.59 |  |
| Hampton Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 7 | Signal | 38.4 | 7.8 | II | 23.4 | c | 0.59 |  |
| Ferncreek Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 7 | Signal | 21.6 | 3.0 | 11 | 23.3 | C | 0.58 |  |
| Shine Ave to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 581 | 7 | Signal | 27.6 | 13.2 | II | 14.3 | E | 0.36 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,837 |  |  | 343.2 | 42.0 | II | 29.5 | B | 0.74 | 0.097 galveh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 5 | Signal | 55.8 | 23.4 | 11 | 11.0 | F | 0.24 |  |
| Old Cheney Hwy to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 5 | Signal | 32.4 | 0.0 | II | 44.4 | A | 0.99 |  |
| Humphries Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 5 | Signal | 57.0 | 18.6 | II | 22.7 | C | 0.51 |  |
| Bennett Rd to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 5 | Signal | 25.8 | 0.0 | 11 | 34.9 | B | 0.87 |  |
| Herndon Ave to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 5 | Signal | 10.2 | 0.0 | II | 42.4 | A | 1.06 |  |
| Fashion Square Mall Entrance to Maguire Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,690 | 5 | Signal | 48.0 | 15.6 | II | 24.0 | C | 0.60 |  |
| Maguire Blvd to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,003 | 5 | Signal | 47.4 | 18.0 | " | 14.4 | E | 0.36 |  |
| Primerose Dr to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 5 | Signal | 15.6 | 0.0 | 11 | 30.0 | B | 0.75 |  |
| Coy St to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 634 | 5 | Signal | 12.6 | 0.0 | II | 34.3 | B | 0.86 |  |
| Bumby Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 34.2 | 4.2 | II | 26.3 | C | 0.66 |  |
| Hampton Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 51.6 | 17.4 | II | 17.4 | D | 0.44 |  |
| Ferncreek Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 5 | Signal | 21.0 | 0.0 | II | 24.0 | C | 0.60 |  |
| Shine Ave to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 581 | 5 | Signal | 83.4 | 66.6 | II | 4.7 | F | 0.12 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,837 |  |  | 495.0 | 163.8 | II | 20.4 | D | 0.51 | $0.101 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 11

Year 2010 METROPLAN Orlando Travel Time Study
SR 50 - Eastbound Direction Summary - Before Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{aligned} & \text { Area } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | Roadway <br> Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 5 | Signal | 33.0 | 21.6 | II | 6.5 | F | 0.16 |  |
| Mills Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 581 | 5 | Signal | 15.6 | 1.8 | II | 25.4 | C | 0.63 |  |
| Shine Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 5 | Signal | 35.4 | 15.6 | II | 14.2 | E | 0.36 |  |
| Ferncreek Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 29.4 | 1.8 | II | 30.6 | B | 0.77 |  |
| Hampton Ave to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 5 | Signal | 47.4 | 15.6 | II | 19.0 | D | 0.47 |  |
| Bumby Ave to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 5 | Signal | 13.2 | 0.0 | II | 32.7 | B | 0.82 |  |
| Coy St to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 5 | Signal | 56.4 | 36.0 | II | 8.3 | F | 0.21 |  |
| Primerose Dr to Maguire Blva | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,003 | 5 | Signal | 22.2 | 0.0 | II | 30.8 | B | 0.77 |  |
| Maguire Blvd to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,690 | 5 | Signal | 49.8 | 7.2 | II | 23.1 | C | 0.58 |  |
| Fashion Square Mall Entrance to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 634 | 5 | Signal | 42.0 | 22.8 | II | 10.3 | F | 0.26 |  |
| Herndon Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,320 | 5 | Signal | 23.4 | 0.0 | II | 38.5 | A | 0.85 |  |
| Bennett Rd to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 5 | Signal | 35.4 | 2.4 | 11 | 36.6 | A | 0.81 |  |
| Humphries Ave to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 5 | Signal | 34.2 | 0.6 | II | 42.1 | A | 0.94 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,256 |  |  | 437.4 | 125.4 | II | 22.2 | C | 0.56 | 0.097 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 317 | 4 | Signal | 44.4 | 31.8 | 11 | 4.9 | F | 0.12 |  |
| Mills Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 581 | 4 | Signal | 13.2 | 0.0 | 1 | 30.0 | B | 0.75 |  |
| Shine Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 4 | Signal | 19.8 | 1.2 | II | 25.5 | C | 0.64 |  |
| Ferncreek Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 4 | Signal | 47.4 | 13.2 | 11 | 19.0 | D | 0.47 |  |
| Hampton Ave to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 4 | Signal | 62.4 | 26.4 | II | 14.4 | E | 0.36 |  |
| Bumby Ave to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 4 | Signal | 15.6 | 0.0 | II | 27.7 | C | 0.69 |  |
| Coy St to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 4 | Signal | 31.2 | 16.8 | II | 15.0 | E | 0.37 |  |
| Primerose Dr to Maguire Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,003 | 4 | Signal | 28.2 | 4.8 | II | 24.3 | C | 0.61 |  |
| Maguire Blvd to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,690 | 4 | Signal | 72.0 | 27.0 | II | 16.0 | E | 0.40 |  |
| Fashion Square Mall Entrance to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 634 | 4 | Signal | 42.6 | 25.2 | II | 10.1 | F | 0.25 |  |
| Herndon Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,320 | 4 | Signal | 39.6 | 11.4 | II | 22.7 | C | 0.51 |  |
| Bennett Rd to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 4 | Signal | 54.0 | 15.0 | 11 | 24.0 | C | 0.53 |  |
| Humphries Ave to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 48.0 | 10.8 | II | 30.0 | B | 0.67 |  |
| total |  |  |  |  |  |  | 40 | 14,256 |  |  | 518.4 | 183.6 | II | 18.7 | D | 0.47 | $0.099 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 11

Year 2010 METROPLAN Orlando Travel Time Study
SR 50 - Westbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadwa | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 5 | Signal | 40.8 | 12.6 | II | 15.0 | E | 0.33 |  |
| Old Cheney Hwy to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 5 | Signal | 58.2 | 15.0 | II | 24.7 | C | 0.55 |  |
| Humphries Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 5 | Signal | 58.8 | 18.0 | II | 22.0 | C | 0.49 |  |
| Bennett Rd to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 5 | Signal | 34.8 | 5.4 | II | 25.9 | C | 0.65 |  |
| Herndon Ave to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 5 | Signal | 12.0 | 0.0 | II | 36.0 | A | 0.90 |  |
| Fashion Square Mall Entrance to Maguire Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,690 | 5 | Signal | 40.2 | 3.0 | II | 28.7 | B | 0.72 |  |
| Maguire Blvd to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,003 | 5 | Signal | 20.4 | 0.0 | II | 33.5 | B | 0.84 |  |
| Primerose Dr to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 5 | Signal | 24.0 | 5.4 | II | 19.5 | D | 0.49 |  |
| Coy St to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 634 | 5 | Signal | 26.4 | 12.0 | II | 16.4 | E | 0.41 |  |
| Bumby Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 49.8 | 15.0 | II | 18.1 | D | 0.45 |  |
| Hampton Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 5 | Signal | 51.6 | 21.6 | II | 17.4 | D | 0.44 |  |
| Ferncreek Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 5 | Signal | 17.4 | 0.0 | 11 | 29.0 | B | 0.72 |  |
| Shine Ave to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 581 | 5 | Signal | 43.2 | 27.6 | II | 9.2 | F | 0.23 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,837 |  |  | 477.6 | 135.6 | II | 21.2 | D | 0.53 | 0.101 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Old Cheney Hwy | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 898 | 4 | Signal | 58.2 | 31.8 | 11 | 10.5 | F | 0.23 |  |
| Old Cheney Hwy to Humphries Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 2,112 | 4 | Signal | 63.0 | 21.0 | " | 22.9 | C | 0.51 |  |
| Humphries Ave to Bennett Rd | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 45 | 1,901 | 4 | Signal | 34.8 | 0.0 | 11 | 37.2 | A | 0.83 |  |
| Bennett Rd to Herndon Ave | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,320 | 4 | Signal | 27.6 | 1.2 | II | 32.6 | B | 0.82 |  |
| Herndon Ave to Fashion Square Mall Entrance | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 1 | 40 | 634 | 4 | Signal | 12.6 | 0.0 | II | 34.3 | B | 0.86 |  |
| Fashion Square Mall Entrance to Maguire Blvd | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 1,690 | 4 | Signal | 74.4 | 25.2 | II | 15.5 | E | 0.39 |  |
| Maguire Blvd to Primerose Dr | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 1,003 | 4 | Signal | 22.8 | 0.0 | II | 30.0 | B | 0.75 |  |
| Primerose Dr to Coy St | City of Orlando | Arterial | Outlying Business District | 1 | 3 | 0 | 40 | 686 | 4 | Signal | 57.0 | 37.2 | 11 | 8.2 | F | 0.21 |  |
| Coy St to Bumby Ave | City of Orlando | Arterial | Outlying Business District | 2 | 3 | 0 | 40 | 634 | 4 | Signal | 31.2 | 13.8 | 11 | 13.8 | E | 0.35 |  |
| Bumby Ave to Hampton Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 4 | Signal | 27.6 | 0.0 | II | 32.6 | B | 0.82 |  |
| Hampton Ave to Ferncreek Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 1,320 | 4 | Signal | 54.0 | 25.2 | II | 16.7 | E | 0.42 |  |
| Ferncreek Ave to Shine Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 0 | 40 | 739 | 4 | Signal | 34.8 | 15.0 | 1 | 14.5 | E | 0.36 |  |
| Shine Ave to Mills Ave | City of Orlando | Arterial | Outlying Business District | 1 | 2 | 1 | 40 | 581 | 4 | Signal | 43.8 | 26.4 | II | 9.0 | F | 0.23 |  |
| TOTAL |  |  |  |  |  |  | 40 | 14,837 |  |  | 541.8 | 196.8 | II | 18.7 | D | 0.47 | $0.101 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) M <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




SR 50 : Mills Ave to Old Cheney Hwy: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1082 | 437.4 | 125.4 | 22.2 | 0.0970 | 131.46 | 104.95 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2017 | 518.4 | 183.6 | 18.7 | 0.0990 | 290.45 | 199.68 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2106 | 477.6 | 135.6 | 21.2 | 0.1010 | 279.40 | 212.71 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1762 | 541.8 | 196.8 | 18.7 | 0.1010 | 265.18 | 177.96 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 50 : Mills Ave to Old Cheney Hwy: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1082 | 387.0 | 101.4 | 25.1 | 0.0940 | 116.32 | 101.71 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2017 | 408.0 | 97.8 | 23.8 | 0.0980 | 228.59 | 197.67 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2106 | 343.2 | 42.0 | 29.5 | 0.0970 | 200.77 | 204.28 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1762 | 495.0 | 163.8 | 20.4 | 0.1010 | 242.28 | 177.96 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 50 : Mills Ave to Old Cheney Hwy: Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 410.86 | 317.09 | 555.63 | 470.87 |
| Total Fuel Consumption (gallons) | 317.66 | 305.99 | 377.65 | 375.63 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 1,619.52$ | $\$ 1,438.47$ |
| Annual User Benefit | $\$ 485,857.02$ | $\$ 431,539.78$ |
| Total Annual User Benefit $=$ | $\$ 917,396.80$ |  |
| Total Signal Retiming Annual Cost | $\$ 21,948.58$ |  |
| User Benefit Cost Ratio | $\mathbf{4 1 . 8 0}$ |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Mills Ave (SR 15/600)

## Marks St to Lake Shore Dr/Rollins St

## table 12

Year 2010 METROPLAN Orlando Travel Time Study
Mills Ave - Northbound Direction Summary - Before Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes } \end{gathered}$ | Thru Lanes ${ }^{2}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance(ft) | \# Runs | Traffic Control Device | Travel Time (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway <br> Class | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Colonial Dr to Marks St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,267 | 6 | Signal | 30.6 | 1.2 | 11 | 28.2 | B | 0.81 |  |
| Marks St to Lake Highland Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 6 | Signal | 37.2 | 1.2 | 11 | 30.0 | B | 0.86 |  |
| Lake Highland Dr to Virginia Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 64.2 | 36.0 | II | 10.7 | F | 0.30 |  |
| Virginia Dr to Nebraska St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 528 | 6 | Signal | 12.6 | 0.0 | 11 | 28.6 | B | 0.82 |  |
| Nebraska St to Princeton St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | Signal | 37.2 | 0.6 | 11 | 31.0 | B | 0.88 |  |
| Princeton St to Rollins St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 21.0 | 0.0 | 11 | 32.6 | B | 0.93 |  |
| Rollins St to Dorchester St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,531 | 6 | Stop | 28.8 | 0.0 | 11 | 36.2 | A | 1.04 |  |
| TOTAL |  |  |  |  |  |  | 35 | 8,659 |  |  | 231.6 | 39.0 | 11 | 25.5 | C | 0.73 | $0.059 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE C |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Colonial Dr to Marks St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,267 | 6 | Signal | 46.2 | 12.0 | 11 | 18.7 | D | 0.53 |  |
| Marks St to Lake Highland Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 6 | Signal | 38.4 | 3.0 | II | 29.1 | B | 0.83 |  |
| Lake Highland Dr to Virginia Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 76.2 | 46.8 | II | 9.0 | F | 0.26 |  |
| Virginia Dr to Nebraska St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 528 | 6 | Signal | 13.8 | 0.0 | II | 26.1 | C | 0.75 |  |
| Nebraska St to Princeton St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | Signal | 39.6 | 1.2 | II | 29.1 | B | 0.83 |  |
| Princeton St to Rollins St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 21.0 | 0.0 | II | 32.6 | B | 0.93 |  |
| Rollins St to Dorchester St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,531 | 6 | Signal | 28.2 | 0.0 | 11 | 37.0 | A | 1.06 |  |
| TOTAL |  |  |  |  |  |  | 35 | 8,659 |  |  | 263.4 | 63.0 | II | 22.4 | C | 0.64 | $0.060 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 12

Year 2010 METROPLAN Orlando Travel Time Study
Mills Ave - Southbound Direction Summary - Before Condition

|  | Jurisdiction | $\begin{gathered} \text { Facility } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ |  | Thru Lanes ${ }^{2}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dorchester St to Rollins St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,531 | 6 | Signal | 40.8 | 6.0 | 11 | 25.6 | C | 0.73 |  |
| Rollins St to Princeton St | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,003 | 6 | Signal | 33.0 | 5.4 | II | 20.7 | D | 0.59 |  |
| Princeton St to Nebraska St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | Signal | 37.2 | 0.0 | 11 | 31.0 | B | 0.88 |  |
| Nebraska St to Virginia Dr | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 35 | 528 | 6 | Signal | 15.0 | 0.0 | 11 | 24.0 | C | 0.69 |  |
| Virginia Dr to Lake Highland Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 19.8 | 0.0 | 11 | 34.5 | B | 0.99 |  |
| Lake Highland Dr to Marks st | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 6 | Signal | 31.8 | 0.0 | 11 | 35.1 | A | 1.00 |  |
| Marks st to Colonial Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,267 | 6 | Signal | 48.6 | 18.6 | II | 17.8 | D | 0.51 |  |
| TOTAL |  |  |  |  |  |  | 35 | 8,659 |  |  | 226.2 | 30.0 | II | 26.1 | c | 0.75 | 0.059 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dorchester St to Rollins St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,531 | 6 | Signal | 46.2 | 9.6 | 11 | 22.6 | C | 0.65 |  |
| Rollins St to Princeton St | City of Orlando | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,003 | 6 | Signal | 40.2 | 13.8 | 11 | 17.0 | D | 0.49 |  |
| Princeton St to Nebraska St | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | Signal | 48.0 | 6.0 | 11 | 24.0 | C | 0.69 |  |
| Nebraska St to Virginia Dr | City of Orlando | Arterial | Residential Area | 2 | 2 | 0 | 35 | 528 | 6 | Signal | 18.6 | 3.0 | 11 | 19.4 | D | 0.55 |  |
| Virginia Dr to Lake Highland Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,003 | 6 | Signal | 20.4 | 0.0 | 11 | 33.5 | B | 0.96 |  |
| Lake Highland Dr to Marks st | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 6 | Signal | 33.6 | 0.0 | 11 | 33.2 | B | 0.95 |  |
| Marks st to Colonial Dr | City of Orlando | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,267 | 6 | Signal | 66.0 | 31.2 | II | 13.1 | E | 0.37 |  |
| TOTAL |  |  |  |  |  |  | 35 | 8,659 |  |  | 273.0 | 63.6 | 11 | 21.6 | D | 0.62 | $0.060 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## Central Blvd

## Brown Ave to Mills Ave

## TABLE 13

Year 2010 METROPLAN Orlando Travel Time Study
Central Blvd - Eastbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | urn | Limi |  |  | Control | Tim | Delay | Roadway | Avera | Speed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ |  | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CO |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Summerlin Ave to Thornton Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,109 | 6 | Signal | 43.2 | 8.4 | Iv | 17.5 | C | 0.70 |  |
| Thornton Ave to Mills Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 317 | 6 | Stop | 10.2 | 0.0 | Iv | 21.2 | B | 0.85 |  |
| Mills Ave to Brown Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 264 | 6 | Signal | 22.8 | 10.2 | IV | 7.9 | E | 0.32 |  |
| Brown Ave to Ferncreek Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,003 | 6 | Stop | 28.2 | 0.0 | Iv | 24.3 | B | 0.97 |  |
| TOTAL |  |  |  |  |  |  | 25 | 2,693 |  |  | 104.4 | 18.6 | IV | 17.6 | C | 0.70 | $0.020 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - beFore co |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Summerlin Ave to Thornton Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,109 | 6 | Signal | 64.8 | 28.2 | Iv | 11.7 | D | 0.47 |  |
| Thornton Ave to Mills Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 317 | 6 | Stop | 10.2 | 0.0 | Iv | 21.2 | B | 0.85 |  |
| Mills Ave to Brown Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 264 | 6 | Signal | 21.6 | 8.4 | IV | 8.3 | E | 0.33 |  |
| Brown Ave to Ferncreek Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,003 | 6 | Stop | 25.8 | 0.6 | v | 26.5 | A | 1.06 |  |
| TOTAL |  |  |  |  |  |  | 25 | 2,693 |  |  | 122.4 | 37.2 | IV | 15.0 | C | 0.60 | $0.020 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
.The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 13

Year 2010 METROPLAN Orlando Travel Time Study
Central Blvd - Westbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadw | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | urn | Limi | Distance |  | Control | Tim | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ |  | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CO |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Ferrcreek Ave to Brown Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,003 | 6 | Signal | 39.0 | 12.6 | Iv | 17.5 | C | 0.70 |  |
| Brown Ave to Mills Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 264 | 6 | Stop | 12.6 | 0.0 | Iv | 14.3 | c | 0.57 |  |
| Mills Ave to Thornton Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 317 | 6 | Signal | 28.2 | 15.6 | IV | 7.7 | E | 0.31 |  |
| Thornton Ave to Summerlin Ave | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 25 | 1,109 | 6 | Signal | 48.0 | 12.0 | Iv | 15.7 | C | 0.63 |  |
| TOTAL |  |  |  |  |  |  | 25 | 2,693 |  |  | 127.8 | 40.2 | IV | 14.4 | c | 0.57 | 0.021 gal/veh |
| PM PEAK HOUR - before co |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Ferncreek Ave to Brown Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 1,003 | 6 | Signal | 26.4 | 0.6 | IV | 25.9 | A | 1.04 |  |
| Brown Ave to Mills Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 264 | 6 | Stop | 9.0 | 0.0 | Iv | 20.0 | B | 0.80 |  |
| mills Ave to Thornton Ave | City of Orlando | Arterial | Residential Area | 0 | 1 | 0 | 25 | 317 | 6 | Signal | 21.0 | 10.8 | iv | 10.3 | D | 0.41 |  |
| Thornton Ave to Summerlin Ave | City of Orlando | Arterial | Residential Area | 1 | 1 | 0 | 25 | 1,109 | 6 | Signal | 56.4 | 22.2 | iv | 13.4 | C | 0.54 |  |
| TOTAL |  |  |  |  |  |  | 25 | 2,693 |  |  | 112.8 | 33.6 | IV | 16.3 | c | 0.65 | $0.020 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel

## US 17/92

## Osceola Pkwy to Columbia Ave

## TABLE 14

Year 2010 METROPLAN Orlando Travel Time Study
US 17/92 (Osceola) - Northbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | Left Turn Lanes ${ }^{2}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right <br> Turn <br> Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | $\begin{gathered} \text { Roadway Segment } \\ \hline \text { Average Speed } \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 264 | 8 | Signal | 11.4 | 3.6 | II | 15.8 | E | 0.35 |  |
| Columbia Ave to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,643 | 8 | Signal | 73.8 | 7.2 | 11 | 33.7 | B | 0.75 |  |
| Donegan Ave to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 8 | Signal | 61.2 | 12.0 | II | 29.4 | B | 0.65 |  |
| Carroll St to Osceola Pkwy | Osceola | Arterial | Residential Area | 2 | 3 | 1 | 45 | 3,960 | 8 | Signal | 96.6 | 28.8 | II | 27.9 | C | 0.62 |  |
| TOTAL |  |  |  |  |  |  | 45 | 10,507 |  |  | 243.0 | 51.6 | II | 29.5 | B | 0.66 | 0.070 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 264 | 7 | Signal | 12.0 | 5.4 | II | 15.0 | E | 0.33 |  |
| Columbia Ave to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,643 | 7 | Signal | 78.6 | 13.2 | II | 31.6 | B | 0.70 |  |
| Donegan Ave to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 7 | Signal | 72.0 | 19.2 | II | 25.0 | C | 0.56 |  |
| Carroll St to Osceola Pkwy | Osceola | Arterial | Residential Area | 2 | 3 | 1 | 45 | 3,960 | 7 | Signal | 106.8 | 32.4 | II | 25.3 | C | 0.56 |  |
| TOTAL |  |  |  |  |  |  | 45 | 10,507 |  |  | 269.4 | 70.2 | II | 26.6 | C | 0.59 | 0.070 gal/veh |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 14

Year 2010 METROPLAN Orlando Travel Time Study
US 17/92 (Osceola) - Southbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | Left Turn Lanes ${ }^{2}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right <br> Turn <br> Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Contro <br> Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Osceola Pkwy | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,214 | 6 | Signal | 46.8 | 21.6 | 11 | 17.7 | D | 0.39 |  |
| Osceola Pkwy to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 3,960 | 6 | Signal | 84.0 | 14.4 | 11 | 32.1 | B | 0.71 |  |
| Carroll St to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 6 | Signal | 46.2 | 0.0 | 11 | 39.0 | A | 0.87 |  |
| Donegan Ave to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 40 | 3,643 | 6 | Signal | 61.2 | 2.4 | 11 | 40.6 | A | 1.01 |  |
| total |  |  |  |  |  |  | 45 | 11,458 |  |  | 238.2 | 38.4 | II | 32.8 | B | 0.73 | $0.083 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Osceola Pkwy | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,214 | 5 | Signal | 58.8 | 27.0 | 11 | 14.1 | E | 0.31 |  |
| Osceola Pkwy to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 3,960 | 5 | Signal | 93.0 | 21.6 | 11 | 29.0 | B | 0.65 |  |
| Carroll St to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 5 | Signal | 43.8 | 0.0 | 11 | 41.1 | A | 0.91 |  |
| Donegan Ave to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 40 | 3,643 | 5 | Signal | 65.4 | 3.6 | 11 | 38.0 | A | 0.95 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,458 |  |  | 261.0 | 52.2 | II | 29.9 | B | 0.67 | $0.084 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 14

Year 2010 METROPLAN Orlando Travel Time Study
US 17/92 (Osceola) - Northbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | $\begin{gathered} \text { Facility }{ }^{\text {Type }^{1}} \\ \hline \end{gathered}$ | Area Type ${ }^{1}$ | Left <br> Turn <br> Lanes ${ }^{2}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 264 | 9 | Signal | 8.4 | 3.6 | II | 21.4 | D | 0.48 |  |
| Columbia Ave to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,643 | 9 | Signal | 60.0 | 1.2 | 11 | 41.4 | A | 0.92 |  |
| Donegan Ave to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 9 | Signal | 42.6 | 1.8 | II | 42.3 | A | 0.94 |  |
| Carroll St to Osceola Pkwy | Osceola | Arterial | Residential Area | 2 | 3 | 1 | 45 | 3,960 | 9 | Signal | 68.4 | 9.0 | II | 39.5 | A | 0.88 |  |
| TOTAL |  |  |  |  |  |  | 45 | 10,507 |  |  | 179.4 | 15.6 | II | 39.9 | A | 0.89 | $0.067 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 264 | 7 | Signal | 7.8 | 3.0 | II | 23.1 | C | 0.51 |  |
| Columbia Ave to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,643 | 7 | Signal | 64.8 | 9.6 | II | 38.3 | A | 0.85 |  |
| Donegan Ave to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 7 | Signal | 73.8 | 21.0 | II | 24.4 | C | 0.54 |  |
| Carroll St to Osceola Pkwy | Osceola | Arterial | Residential Area | 2 | 3 | 1 | 45 | 3,960 | 7 | Signal | 72.0 | 12.0 | 11 | 37.5 | A | 0.83 |  |
| total |  |  |  |  |  |  | 45 | 10,507 |  |  | 218.4 | 45.6 | II | 32.8 | B | 0.73 | 0.068 gal/veh |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 14

Year 2010 METROPLAN Orlando Travel Time Study
US 17/92 (Osceola) - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right <br> Turn <br> Lanes ${ }^{2}$ | Speed Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{gathered} \hline \text { Travel } \\ \text { Time } \\ \text { (sec) } \end{gathered}$ |  | Roadway Class | Roadway SegmentAverage Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ <br> Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Osceola Pkwy | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,214 | 9 | Signal | 44.4 | 17.4 | II | 18.6 | D | 0.41 |  |
| Osceola Pkwy to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 3,960 | 9 | Signal | 61.2 | 0.0 | 11 | 44.1 | A | 0.98 |  |
| Carroll St to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 9 | Signal | 45.6 | 0.0 | 11 | 39.5 | A | 0.88 |  |
| Donegan Ave to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 40 | 3,643 | 9 | Signal | 57.0 | 1.2 | 11 | 43.6 | A | 1.09 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,458 |  |  | 208.2 | 18.6 | II | 37.5 | A | 0.83 | $0.082 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Osceola Pkwy | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,214 | 7 | Signal | 57.0 | 30.0 | 11 | 14.5 | E | 0.32 |  |
| Osceola Pkwy to Carroll St | Osceola | Arterial | Residential Area | 1 | 2 | 1 | 45 | 3,960 | 7 | Signal | 68.4 | 4.2 | 11 | 39.5 | A | 0.88 |  |
| Carroll St to Donegan Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,640 | 7 | Signal | 42.6 | 0.0 | 11 | 42.3 | A | 0.94 |  |
| Donegan Ave to Columbia Ave | Osceola | Arterial | Residential Area | 1 | 2 | 0 | 40 | 3,643 | 7 | Signal | 57.6 | 1.2 | II | 43.1 | A | 1.08 |  |
| TOTAL |  |  |  |  |  |  | 45 | 11,458 |  |  | 225.6 | 35.4 | 11 | 34.6 | B | 0.77 | $0.082 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.





2010 MEIROPLAN ORLANDO
Travel Time Study

US 17-92 : Osceola Pkwy to Columbia Ave: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 813 | 243.0 | 51.6 | 29.5 | 0.0700 | 54.88 | 56.91 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 904 | 269.4 | 70.2 | 26.6 | 0.0700 | 67.65 | 63.28 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1117 | 238.2 | 38.4 | 32.8 | 0.0830 | 73.91 | 92.71 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1708 | 261.0 | 52.2 | 29.9 | 0.0840 | 123.83 | 143.47 |

*Traffic Volumes are obtained from the latest FDOT Counts

US 17-92 : Osceola Pkwy to Columbia Ave: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 813 | 179.4 | 15.6 | 39.9 | 0.0670 | 40.51 | 54.47 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 904 | 218.4 | 45.6 | 32.8 | 0.0680 | 54.84 | 61.47 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1117 | 208.2 | 18.6 | 37.5 | 0.0820 | 64.60 | 91.59 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1708 | 225.6 | 35.4 | 34.6 | 0.0820 | 107.03 | 140.06 |

*Traffic Volumes are obtained from the latest FDOT Counts

US 17-92 : Osceola Pkwy to Columbia Ave: Before \& After Study Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 128.79 | 105.11 | 191.48 | 161.88 |
| Total Fuel Consumption (gallons) | 149.62 | 146.07 | 206.75 | 201.53 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 410.64$ | $\$ 515.84$ <br> Annual User Benefit |
| $\$ 123,192.72$ | $\$ 277,945.12$ |  |
| Total Annual User Benefit $=$ | $\$ 6,439.77$ |  |
| Total Signal Retiming Annual Cost | $\mathbf{4 3 . 1 6}$ |  |
| User Benefit / Cost Ratio |  |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

US 192

## Denn John Ln to Turnpike NB Ramp

## TABLE 15

Year 2010 METROPLAN Orlando Travel Time Study
US 192 - Eastbound Direction Summary - Before Condition

| Roadway <br> Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Type }^{1}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Thru } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Denn John Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 739 | 4 | Signal | 13.2 | 1.8 | 1 | 38.2 | B | 0.76 |  |
| Denn John Ln to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 2 | 3 | 0 | 50 | 1,848 | 4 | Signal | 24.6 | 0.0 | 1 | 51.2 | A | 1.02 |  |
| Boggy Creek Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 4 | Signal | 56.4 | 0.0 | 1 | 49.1 | A | 0.98 |  |
| Bill Beck Blvd to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 4 | Signal | 77.4 | 16.2 | 1 | 33.5 | c | 0.67 |  |
| Simpson Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,637 | 4 | Signal | 43.8 | 14.4 | 1 | 25.5 | D | 0.51 |  |
| Shady Ln to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 2 | 2 | 0 | 55 | 3,115 | 4 | Signal | 53.4 | 6.6 | 1 | 39.8 | B | 0.72 |  |
| Partin Settlement Rd to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 4 | Signal | 88.2 | 0.0 | 1 | 53.1 | A | 0.96 |  |
| Turnpike Ramp to Median Opening | Osceola | Arterial | Outlying Business District | 2 | 2 | 1 | 55 | 475 | 4 | Signal | 5.6 | 0.0 | 1 | 57.9 | A | 1.05 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 362.6 | 39.0 | 1 | 42.4 | A | 0.85 | $0.145 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Denn John Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 739 | 4 | Signal | 16.2 | 5.4 | 1 | 31.1 | c | 0.62 |  |
| Denn John Ln to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 2 | 3 | 0 | 50 | 1,848 | 4 | Signal | 24.0 | 0.0 | 1 | 52.5 | A | 1.05 |  |
| Boggy Creek Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 4 | Signal | 63.6 | 10.2 | 1 | 43.6 | A | 0.87 |  |
| Bill Beck Blvd to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 4 | Signal | 56.4 | 0.6 | 1 | 46.0 | A | 0.92 |  |
| Simpson Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,637 | 4 | Signal | 51.6 | 20.4 | 1 | 21.6 | D | 0.43 |  |
| Shady Ln to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 2 | 2 | 0 | 55 | 3,115 | 4 | Signal | 70.8 | 11.4 | 1 | 30.0 | C | 0.55 |  |
| Partin Settlement Rd to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 4 | Signal | 92.4 | 2.4 | 1 | 50.6 | A | 0.92 |  |
| Turnpike Ramp to Median Opening | Osceola | Arterial | Outlying Business District | 2 | 2 | 1 | 55 | 475 | 4 | Signal | 5.4 | 0.0 | 1 | 60.0 | A | 1.09 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 380.4 | 50.4 | 1 | 40.4 | B | 0.81 | $0.146 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 15

Year 2010 METROPLAN Orlando Travel Time Study
US 192 - Westbound Direction Summary - Before Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadwa | egment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 475 | 4 | Signal | 9.0 | 1.2 | 1 | 36.0 | B | 0.65 |  |
| Turnpike Ramp to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 4 | Signal | 108.0 | 17.4 | 1 | 43.3 | A | 0.79 |  |
| Partin Settlement Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 55 | 3,115 | 4 | Signal | 54.6 | 1.8 | 1 | 38.9 | B | 0.71 |  |
| Shady Ln to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,637 | 4 | Signal | 60.0 | 22.8 | 1 | 18.6 | E | 0.37 |  |
| Simpson Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 4 | Signal | 64.2 | 1.8 | 1 | 40.4 | B | 0.81 |  |
| Bill Beck Blvd to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 4 | Signal | 64.2 | 3.0 | 1 | 43.2 | A | 0.86 |  |
| Boggy Creek Rd to Denn John Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,848 | 4 | Signal | 33.0 | 1.2 | 1 | 38.2 | B | 0.76 |  |
| Denn John Ln to Median Opening | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 739 | 4 | Signal | 10.2 | 0.0 | 1 | 49.4 | A | 0.99 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 403.2 | 49.2 | 1 | 38.1 | B | 0.76 | $0.146 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 475 | 4 | Signal | 7.2 | 0.0 | 1 | 45.0 | A | 0.82 |  |
| Turnpike Ramp to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 4 | Signal | 108.6 | 19.8 | 1 | 43.1 | A | 0.78 |  |
| Partin Settlement Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 55 | 3,115 | 4 | Signal | 81.0 | 28.2 | 1 | 26.2 | D | 0.48 |  |
| Shady Ln to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,637 | 4 | Signal | 77.4 | 43.2 | 1 | 14.4 | F | 0.29 |  |
| Simpson Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 4 | Signal | 65.4 | 6.6 | 1 | 39.6 | B | 0.79 |  |
| Bill Beck Blvd to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 4 | Signal | 56.4 | 0.6 | 1 | 49.1 | A | 0.98 |  |
| Boggy Creek Rd to Denn John Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,848 | 4 | Signal | 24.0 | 0.0 | 1 | 52.5 | A | 1.05 |  |
| Denn John Ln to Median Opening | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 739 | 4 | Signal | 9.1 | 0.0 | 1 | 55.4 | A | 1.11 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 429.1 | 98.4 | 1 | 35.8 | B | 0.72 | $0.148 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 15

Year 2010 METROPLAN Orlando Travel Time Study
US 192 - Eastbound Direction Summary - After Condition

| Roadway <br> Segment | Jurisdiction | Facility <br> Type ${ }^{1}$ | $\begin{aligned} & \text { Area } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Thru } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | $\begin{aligned} & \hline \text { Traffic } \\ & \text { Control } \\ & \text { Device } \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Travel } \\ \text { Time } \\ \text { (sec) } \\ \hline \end{gathered}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Denn John Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 739 | 5 | Signal | 13.8 | 1.8 | 1 | 36.5 | B | 0.73 |  |
| Denn John Ln to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 2 | 3 | 0 | 50 | 1,848 | 5 | Signal | 23.4 | 0.0 | 1 | 53.8 | A | 1.08 |  |
| Boggy Creek Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 5 | Signal | 52.8 | 0.0 | 1 | 52.5 | A | 1.05 |  |
| Bill Beck Blvd to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 5 | Signal | 51.0 | 5.4 | 1 | 50.8 | A | 1.02 |  |
| Simpson Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,637 | 5 | Signal | 42.0 | 15.0 | 1 | 26.6 | D | 0.53 |  |
| Shady Ln to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 2 | 2 | 0 | 55 | 3,115 | 5 | Signal | 49.8 | 1.2 | 1 | 42.6 | A | 0.78 |  |
| Partin Settlement Rd to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 5 | Signal | 84.6 | 2.4 | 1 | 55.3 | A | 1.01 |  |
| Turnpike Ramp to Median Opening | Osceola | Arterial | Outlying Business District | 2 | 2 | 1 | 55 | 475 | 5 | Signal | 6.6 | 0.0 | 1 | 49.1 | A | 0.89 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 324.0 | 25.8 | 1 | 47.4 | A | 0.95 | 0.144 galveh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Denn John Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 739 | 5 | Signal | 16.8 | 3.6 | 1 | 30.0 | c | 0.60 |  |
| Denn John Ln to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 2 | 3 | 0 | 50 | 1,848 | 5 | Signal | 27.0 | 0.0 | 1 | 46.7 | A | 0.93 |  |
| Boggy Creek Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 5 | Signal | 50.4 | 0.0 | 1 | 55.0 | A | 1.10 |  |
| Bill Beck Blvd to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 5 | Signal | 51.6 | 11.4 | 1 | 50.2 | A | 1.00 |  |
| Simpson Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 1,637 | 5 | Signal | 34.8 | 3.0 | 1 | 32.1 | c | 0.64 |  |
| Shady Ln to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 2 | 2 | 0 | 55 | 3,115 | 5 | Signal | 57.6 | 0.6 | 1 | 36.9 | B | 0.67 |  |
| Partin Settlement Rd to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 5 | Signal | 90.6 | 0.0 | 1 | 51.7 | A | 0.94 |  |
| Turnpike Ramp to Median Opening | Osceola | Arterial | Outlying Business District | 2 | 2 | 1 | 55 | 475 | 5 | Signal | 19.2 | 12.0 | 1 | 16.9 | E | 0.31 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 348.0 | 30.6 | 1 | 44.2 | A | 0.88 | $0.145 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 15

Year 2010 METROPLAN Orlando Travel Time Study
US 192 - Westbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadwa | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | Turn | Limit | Distance |  | Control | Time | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ |  |  | (mph) | (ft) | \# Runs | Device | (sec) | ( sec ) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 475 | 5 | Signal | 7.2 | 0.0 | 1 | 45.0 | A | 0.82 |  |
| Turnpike Ramp to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 5 | Signal | 96.0 | 18.0 | 1 | 48.7 | A | 0.89 |  |
| Partin Settlement Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 55 | 3,115 | 5 | Signal | 54.0 | 1.8 | 1 | 39.3 | B | 0.72 |  |
| Shady Ln to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,637 | 5 | Signal | 37.2 | 7.8 | 1 | 30.0 | C | 0.60 |  |
| Simpson Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 5 | Signal | 54.6 | 0.0 | 1 | 47.5 | A | 0.95 |  |
| Bill Beck Blvd to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 5 | Signal | 53.4 | 0.0 | 1 | 51.9 | A | 1.04 |  |
| Boggy Creek Rd to Denn John Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,848 | 5 | Signal | 33.0 | 3.6 | 1 | 38.2 | B | 0.76 |  |
| Denn John Ln to Median Opening | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 739 | 5 | Signal | 9.6 | 0.0 | 1 | 52.5 | A | 1.05 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 345.0 | 31.2 | 1 | 44.6 | A | 0.89 | $0.146 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Turnpike Ramp | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 475 | 5 | Signal | 9.6 | 0.6 | 1 | 33.7 | C | 0.61 |  |
| Turnpike Ramp to Partin Settlement Rd | Osceola | Arterial | Outlying Business District | 1 | 2 | 1 | 55 | 6,864 | 5 | Signal | 95.4 | 13.8 | 1 | 49.1 | A | 0.89 |  |
| Partin Settlement Rd to Shady Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 55 | 3,115 | 5 | Signal | 52.8 | 7.8 | 1 | 40.2 | B | 0.73 |  |
| Shady Ln to Simpson Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,637 | 5 | Signal | 24.0 | 0.0 | 1 | 46.5 | A | 0.93 |  |
| Simpson Rd to Bill Beck Blvd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 3,802 | 5 | Signal | 50.4 | 0.0 | 1 | 51.4 | A | 1.03 |  |
| Bill Beck Blvd to Boggy Creek Rd | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 4,066 | 5 | Signal | 52.8 | 0.0 | 1 | 52.5 | A | 1.05 |  |
| Boggy Creek Rd to Denn John Ln | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,848 | 5 | Signal | 36.0 | 4.2 | 1 | 35.0 | B | 0.70 |  |
| Denn John Ln to Median Opening | Osceola | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 739 | 5 | Signal | 10.8 | 0.0 | 1 | 46.7 | A | 0.93 |  |
| TOTAL |  |  |  |  |  |  | 50 | 22,546 |  |  | 331.8 | 26.4 | 1 | 46.3 | A | 0.93 | $0.147 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## US 192

- AM Peak

Before Condition
Date of Collection: 2/10/2010 Distance: 4.27 miles From: Denn John Ln. To: Turnpike NB Ramp.

Start Time: 7:45 AM End Time: 9:00 AM

EB Avg Speed: 42.4 MPH EB Travel Time: 6.04 MIN EB Delay Time: 0.65 MIN

WB Avg Speed: 38.1 MPH WB Travel Time: 6.72 MIN WB Delay Time: 0.82 MIN


## US 192

- AM Peak

After Condition
Date of Collection: 6/8/2010 Distance: 4.27 miles From: Denn John Ln. To: Turnpike NB Ramp.
Start Time: 7:45 AM End Time: 9:00 AM

EB Avg Speed: 47.4 MPH EB Avg Speed:
EB Travel Time:
5.40 MIN EB Delay Time: 0.43 MIN

WB Avg Speed: 44.6 MPH WB Travel Time: 5.75 MIN WB Delay Time: 0.52 MIN



2010 MEIROPLAN ORLANDO
Travel Time Study

## US 192

- PM Peak

Before Condition
Date of Collection: 1/28/2010 Distance: 4.27 miles From: Denn John Ln. To: Turnpike NB Ramp

Start Time: 4:45 PM End Time: 6:00 PM

EB Avg Speed: 40.4 MPH EB Travel Time: 6.34 MIN EB Delay Time: 0.84 MIN

WB Avg Speed: 35.8 MPH WB Travel Time: 7.15 MIN WB Delay Time: 1.64 MIN


US 192

- PM Peak

After Condition
Date of Collection: 6/8/2010 Distance: 4.27 miles From: Denn John Ln. To: Turnpike NB Ramp.
Start Time: 4:45 PM End Time: 6:00 PM EB Avg Speed: 44.2 MPH
EB Travel Time: 5.80 MIN EB Delay Time: 0.51 MIN

WB Avg Speed: 46.3 MPH WB Travel Time: 5.53 MIN WB Delay Time: 0.44 MIN



2010 MEIROPLAN ORLANDO
Travel Time Study

US 192 : Denn John Ln to Turnpike NB Ramp: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 883 | 362.6 | 39.0 | 42.4 | 0.1450 | 88.94 | 128.04 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1614 | 380.4 | 50.4 | 40.4 | 0.1460 | 170.55 | 235.64 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1434 | 403.2 | 49.2 | 38.1 | 0.1460 | 160.61 | 209.36 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1098 | 429.1 | 98.4 | 35.8 | 0.1480 | 130.88 | 162.50 |

*Traffic Volumes are obtained from the latest FDOT Counts

US 192 : Denn John Ln to Turnpike NB Ramp: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 883 | 324.0 | 25.8 | 47.4 | 0.1440 | 79.47 | 127.15 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1614 | 348.0 | 30.6 | 44.2 | 0.1450 | 156.02 | 234.03 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1434 | 345.0 | 31.2 | 44.6 | 0.1460 | 137.43 | 209.36 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1098 | 331.8 | 26.4 | 46.3 | 0.1470 | 101.20 | 161.41 |

*Traffic Volumes are obtained from the latest FDOT Counts

## US 192 : Denn John Ln to Turnpike NB Ramp: Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 249.55 | 216.90 | 301.42 | 257.22 |
| Total Fuel Consumption (gallons) | 337.40 | 336.52 | 398.15 | 395.44 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 554.43$ | $\$ 755.10$ |
| Annual User Benefit | $\$ 166,328.56$ | $\$ 226,531.20$ |
| Total Annual User Benefit $=$ | $\$ 392,859.77$ |  |
| Total Signal Retiming Annual Cost | $\$ 11,660.18$ |  |
| User Benefit/ Cost Ratio |  |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## US 17/92

SR 46 (1st St) to 3rd St

## TABLE 16

Year 2010 METROPLAN Orlando Travel Time Study US 17/92 (Seminole)- Northbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | $\begin{array}{\|l\|} \hline \text { Traffic } \\ \text { Control } \\ \text { Device } \\ \hline \end{array}$ | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5th St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 14 | Signal | 27.0 | 9.6 | II | 17.3 | D | 0.43 |  |
| 3rd St. to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 14 | Signal | 19.8 | 4.8 | 11 | 21.8 | D | 0.55 |  |
| SR 46/1st St. to Fulton St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 14 | Stop | 9.6 | 0.0 | II | 41.2 | A | 1.03 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 56.4 | 14.4 | II | 23.0 | C | 0.57 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5th St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 12 | Signal | 50.4 | 30.0 | II | 9.3 | F | 0.23 |  |
| 3rd St. to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 12 | Signal | 22.2 | 5.4 | II | 19.5 | D | 0.49 |  |
| SR 46/1st St. to Fulton St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 12 | Stop | 10.2 | 0.0 | 11 | 38.8 | A | 0.97 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 82.8 | 35.4 | II | 15.7 | E | 0.39 | $0.014 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 16

Year 2010 METROPLAN Orlando Travel Time Study US 17/92 (Seminole)- Southbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility$\text { Type }{ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | Stop <br> Delay <br> (sec) | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fluton St to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 13 | Signal | 32.4 | 18.0 | II | 12.2 | F | 0.31 |  |
| SR 46/1st St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 13 | Signal | 18.6 | 4.2 | II | 23.2 | c | 0.58 |  |
| 3rd St. to 5th St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 13 | Stop | 10.8 | 0.0 | II | 43.3 | A | 1.08 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 61.8 | 22.2 | II | 21.0 | D | 0.52 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fluton St to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 11 | Signal | 48.0 | 29.4 | II | 8.2 | F | 0.21 |  |
| SR 46/1st St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 11 | Signal | 14.4 | 0.0 | 11 | 30.0 | B | 0.75 |  |
| 3rd St. to 5th St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 11 | Stop | 11.4 | 0.0 | II | 41.1 | A | 1.03 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 73.8 | 29.4 | II | 17.6 | D | 0.44 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 16

Year 2010 METROPLAN Orlando Travel Time Study US 17/92 (Seminole)- Northbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | $\begin{array}{\|l\|} \hline \text { Traffic } \\ \text { Control } \\ \text { Device } \\ \hline \end{array}$ | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5th St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 14 | Signal | 18.6 | 4.8 | II | 25.2 | C | 0.63 |  |
| 3rd St. to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 14 | Signal | 13.8 | 0.0 | 11 | 31.3 | B | 0.78 |  |
| SR 46/1st St. to Fulton St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 14 | Stop | 15.0 | 3.0 | II | 26.4 | c | 0.66 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 47.4 | 7.8 | II | 27.3 | c | 0.68 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5th St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 12 | Signal | 13.8 | 0.0 | II | 33.9 | B | 0.85 |  |
| 3rd St. to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 12 | Signal | 16.8 | 3.0 | II | 25.7 | c | 0.64 |  |
| SR 46/1st St. to Fulton St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 12 | Stop | 13.2 | 1.2 | 11 | 30.0 | B | 0.75 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 43.8 | 4.2 | II | 29.6 | B | 0.74 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 16

Year 2010 METROPLAN Orlando Travel Time Study US 17/92 (Seminole)- Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{array}{\|c} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed  <br> $(\mathrm{mph})$ LOS |  |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fluton St to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 15 | Signal | 18.6 | 1.8 | II | 21.3 | D | 0.53 |  |
| SR 46/1st St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 15 | Signal | 12.0 | 0.0 | II | 36.0 | A | 0.90 |  |
| 3rd St. to 5th St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 15 | Stop | 18.6 | 6.0 | II | 25.2 | c | 0.63 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 49.2 | 7.8 | II | 26.3 | c | 0.66 | $0.013 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER COND |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fluton St to SR 46/1st St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 13 | Signal | 27.0 | 12.0 | II | 14.7 | E | 0.37 |  |
| SR 46/1st St. to 3rd St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 634 | 13 | Signal | 13.8 | 1.8 | II | 31.3 | B | 0.78 |  |
| 3rd St. to 5th St. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 686 | 13 | Stop | 15.6 | 1.8 | 11 | 30.0 | в | 0.75 |  |
| TOTAL |  |  |  |  |  |  | 40 | 1,901 |  |  | 56.4 | 15.6 | II | 23.0 | c | 0.57 | $0.013 \mathrm{gal/veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




2010 MEIROPLAN ORLANDO
Travel Time Study
$0 \quad 0.1 \quad 0.2$


Level of Services:

## 2010 MEIROPLAN ORLANDO

Travel Time Study

|  |  |  |  |  |
| :--- | :--- | :--- | :---: | :---: |
| 0 | 0.1 | 0.2 |  |  |

US 17/92 : SR 46 (1st St) to 3rd St: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 824 | 56.4 | 14.4 | 23.0 | 0.0130 | 12.91 | 10.71 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 887 | 82.8 | 35.4 | 15.7 | 0.0140 | 20.40 | 12.42 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 603 | 61.8 | 22.2 | 21.0 | 0.0130 | 10.35 | 7.84 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 863 | 73.8 | 29.4 | 17.6 | 0.0130 | 17.69 | 11.22 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

US 17/92 : SR 46 (1st St) to 3rd St: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 824 | 47.4 | 7.8 | 27.3 | 0.0130 | 10.85 | 10.71 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 887 | 43.8 | 4.2 | 29.6 | 0.0130 | 10.79 | 11.53 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 603 | 49.2 | 7.8 | 26.3 | 0.0130 | 8.24 | 7.84 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 863 | 56.4 | 15.6 | 23.0 | 0.0130 | 13.52 | 11.22 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

US 17/92 : SR 46 (1st St) to 3rd St: Before \& After Study
Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 23.26 | 19.09 | 38.09 | 24.31 |
| Total Fuel Consumption (gallons) | 18.55 | 18.55 | 23.64 | 22.75 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 70.48$ | $\$ 235.53$ |
| Annual User Benefit | $\$ 21,144.44$ | $\$ 70,659.27$ |
| Total Annual User Benefit $=$ | $\$ 91,803.70$ |  |
| Total Signal Retiming Annual Cost | $\$ 3,905.78$ |  |
| User Benefit / Cost Ratio | $\mathbf{2 3 . 5 0}$ |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

SR 434
Sand Lake Rd to Jamestown Blvd

## TABLE 17

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part A - Eastbound Direction Summary - Before Condition


## TABLE 17

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part A - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \end{aligned}$ | Distance <br> (ft) | \# Runs | $\begin{array}{\|c\|} \hline \text { Traffic } \\ \text { Control } \\ \text { Device } \\ \hline \end{array}$ | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | StopDelay(sec) | Roadway Class | $\begin{array}{c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| E. Lake Brantley Rd. to Jamestown Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,534 | 12 | Signal | 57.0 | 7.8 | II | 30.3 | B | 0.67 |  |
| Jamestown Blvd. to Sand Lake Rd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 1,003 | 12 | Signal | 48.0 | 23.4 | 11 | 14.2 | E | 0.32 |  |
| Sand Lake Rd. to San Sebastian Prado | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,904 | 12 | Signal | 47.4 | 0.6 | 11 | 41.8 | A | 0.93 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,442 |  |  | 152.4 | 31.8 | II | 28.8 | B | 0.64 | 0.042 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| E. Lake Brantley Rd. to Jamestown Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,534 | 10 | Signal | 78.0 | 25.8 | II | 22.2 | c | 0.49 |  |
| Jamestown Blvd. to Sand Lake Rd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 1 | 45 | 1,003 | 10 | Signal | 42.6 | 12.6 | II | 16.1 | E | 0.36 |  |
| Sand Lake Rd. to San Sebastian Prado | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,904 | 10 | Signal | 54.6 | 2.4 | 11 | 36.3 | A | 0.81 |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,442 |  |  | 175.2 | 40.8 | II | 25.1 | c | 0.56 | $0.044 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 17

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part A - Eastbound Direction Summary - After Condition


TABLE 17
Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part A - Westbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Thru } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ \text { (mph) } \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | Stop <br> Delay <br> (sec) | $\begin{array}{\|c\|} \text { Roadway } \\ \text { Class } \end{array}$ | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| E. Lake Brantley Rd. to Jamestown Blvd. Jamestown Blvd. to Sand Lake Rd. Sand Lake Rd. to San Sebastian Prado | Seminole <br> Seminole <br> Seminole | Arterial <br> Arterial <br> Arterial | Outlying Business District Outlying Business District Residential Area | $\begin{aligned} & 1 \\ & 1 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2 \\ & 2 \\ & 2 \end{aligned}$ | $\begin{aligned} & 0 \\ & 1 \\ & 0 \end{aligned}$ | $\begin{aligned} & 45 \\ & 45 \\ & 45 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2,534 \\ & 1,003 \\ & 2,904 \\ & \hline \end{aligned}$ | $\begin{aligned} & 10 \\ & 10 \\ & 10 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Signal } \\ & \text { Signal } \\ & \text { Signal } \\ & \hline \end{aligned}$ | $\begin{aligned} & 54.6 \\ & 40.2 \\ & 45.6 \\ & \hline \end{aligned}$ | $\begin{gathered} 9.6 \\ 18.6 \\ 1.2 \\ \hline \end{gathered}$ |  | $\begin{aligned} & 31.6 \\ & 17.0 \\ & 43.4 \end{aligned}$ | B | $\begin{aligned} & 0.70 \\ & 0.38 \\ & 0.96 \end{aligned}$ |  |
| total |  |  |  |  |  |  | 45 | 6,442 |  |  | 140.4 | 29.4 | 11 | 31.3 | B | 0.70 | 0.041 gal/veh |
| PM PEAK HOUR - AFER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| E. Lake Brantley Rd. to Jamestown Blvd. Jamestown Blvd. to Sand Lake Rd. Sand Lake Rd. to San Sebastian Prado | Seminole <br> Seminole <br> Seminole | Arterial Arterial Arterial | Outlying Business District Outlying Business District Residential Area | $\begin{aligned} & 1 \\ & 1 \\ & 1 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2 \\ & 2 \\ & 2 \\ & \hline \end{aligned}$ | $\begin{aligned} & 0 \\ & 1 \\ & 0 \end{aligned}$ | $\begin{aligned} & 45 \\ & 45 \\ & 45 \\ & \hline \end{aligned}$ | $\begin{aligned} & 2,534 \\ & 1,003 \\ & 2,904 \\ & \hline \end{aligned}$ | $\begin{aligned} & 8 \\ & 8 \\ & 8 \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Signal } \\ & \text { Signal } \\ & \text { Signal } \\ & \hline \end{aligned}$ | $\begin{aligned} & 73.8 \\ & 34.8 \\ & 52.2 \\ & \hline \end{aligned}$ | $\begin{gathered} 24.6 \\ 10.8 \\ 6.0 \\ \hline \end{gathered}$ | 11 | $\begin{aligned} & 23.4 \\ & 19.7 \\ & 37.9 \end{aligned}$ | c | $\begin{aligned} & 0.52 \\ & 0.44 \\ & 0.84 \\ & \hline \end{aligned}$ |  |
| TOTAL |  |  |  |  |  |  | 45 | 6,442 |  |  | 160.8 | 41.4 | 11 | 27.3 | C | 0.61 | $0.043 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




| $A$ | $D$ | $\square$ |
| :--- | :--- | :--- |
| $B$ | Roads |  |
| $C$ | $F$ | City Boundary |
| $C$ | Water |  |

SR 434 Part A: Sand Lake Rd to Jamestown Blvd: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 936 | 153.6 | 42.0 | 28.6 | 0.0420 | 39.94 | 39.31 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1430 | 145.8 | 22.8 | 30.1 | 0.0430 | 57.92 | 61.49 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1627 | 152.4 | 31.8 | 28.8 | 0.0420 | 68.88 | 68.33 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1052 | 175.2 | 40.8 | 25.1 | 0.0440 | 51.20 | 46.29 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

SR 434 Part A : Sand Lake Rd to Jamestown Blvd: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 936 | 135.0 | 20.4 | 32.5 | 0.0420 | 35.10 | 39.31 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1430 | 135.0 | 10.8 | 32.5 | 0.0430 | 53.63 | 61.49 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1627 | 140.4 | 29.4 | 31.3 | 0.0410 | 63.45 | 66.71 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1052 | 160.8 | 41.4 | 27.3 | 0.0430 | 46.99 | 45.24 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

## SR 434 Part A : Sand Lake Rd to Jamestown Blvd: Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 108.81 | 98.55 | 109.11 | 100.61 |
| Total Fuel Consumption (gallons) | 107.65 | 106.02 | 107.78 | 106.73 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | \$178.23 | \$146.75 |
| Annual User Benefit | $\$ 53,469.36$ | $\$ 44,025.35$ |
| Total Annual User Benefit $=$ | $\$ 97,494.71$ |  |
| Total Signal Retiming Annual Cost | $\$ 3,861.20$ |  |
| User Benefit / Cost Ratio | 25.25 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## SR 434

## Tollgate Trail to Wayman St

## table 18

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part B - Eastbound Direction Summary - Before Condition

| Roadway <br> Segment | Jurisdiction | $\begin{aligned} & \text { Facility } \\ & \text { Type }^{1} \\ & \hline \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ |  | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ \text { (mph) } \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | $\begin{aligned} & \text { Roadway } \\ & \text { Class } \end{aligned}$ | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 317 | 6 | Signal | 10.8 | 1.8 | II | 20.0 | D | 0.44 |  |
| Tollgate Tr. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 43.2 | 22.2 | II | 14.2 | E | 0.31 |  |
| Palm Springs Dr. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 6 | Signal | 79.2 | 13.8 | II | 25.9 | C | 0.58 |  |
| Rangeline Rd. to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 6 | Signal | 84.0 | 16.8 | II | 24.0 | C | 0.53 |  |
| Florida Central Pkwy. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | NA | 32.4 | 0.0 | 11 | 35.6 | A | 0.89 |  |
| RR Tracks to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 370 | 6 | Signal | 24.0 | 15.0 | II | 10.5 | F | 0.26 |  |
| CR 427/Ronald Reagan Blvd. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,587 | 6 | Signal | 54.6 | 7.2 | II | 32.3 | B | 0.72 |  |
| Grant St. to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 24.6 | 1.2 | II | 36.6 | A | 0.81 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 352.8 | 78.0 | II | 25.4 | C | 0.56 | $0.089 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 317 | 6 | Signal | 6.6 | 0.6 | II | 32.7 | B | 0.73 |  |
| Tollgate Tr. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 39.6 | 20.4 | II | 15.5 | E | 0.34 |  |
| Palm Springs Dr. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 6 | Signal | 61.8 | 3.6 | II | 33.2 | B | 0.74 |  |
| Rangeline Rd. to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 6 | Signal | 85.2 | 18.0 | II | 23.7 | C | 0.53 |  |
| Florida Central Pkwy. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | NA | 127.2 | 75.6 | II | 9.1 | F | 0.23 |  |
| RR Tracks to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 370 | 6 | Signal | 81.6 | 67.8 | 11 | 3.1 | F | 0.08 |  |
| CR 427/Ronald Reagan Blvd. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,587 | 6 | Signal | 47.4 | 0.0 | II | 37.2 | A | 0.83 |  |
| Grant St. to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 19.8 | 0.0 | 11 | 45.5 | A | 1.01 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 469.2 | 186.0 | II | 19.1 | D | 0.42 | $0.090 \mathrm{ga} / \mathrm{veh}$ |

Note.

1. The
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 18

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part B - Westbound Direction Summary - Before Condition

|  |  |  |  | Left |  |  | Speed |  |  | Traffic | Travel | Stop |  | Roadw | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn |  | Turn | Limit | Distance |  | Control | Tim | Delay | Roadway | Avera | peed | Avg Speed/ | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) | (ft) | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 317 | 6 | Signal | 20.4 | 3.0 | II | 10.6 | F | 0.24 |  |
| s. Wayman St. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 32.4 | 4.8 | 11 | 27.8 | C | 0.62 |  |
| Grant St. to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,587 | 6 | Signal | 112.8 | 54.6 | II | 15.6 | E | 0.45 |  |
| CR 427/Ronald Reagan Blvd. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | NA | 7.8 | 0.0 | II | 32.3 | B | 0.81 |  |
| RR Tracks to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,690 | 6 | Signal | 47.4 | 12.0 | 11 | 24.3 | C | 0.61 |  |
| Florida Central Pkwy. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 6 | Signal | 61.2 | 6.0 | II | 32.9 | B | 0.73 |  |
| Rangeline Rd. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 6 | Signal | 61.2 | 0.0 | 11 | 33.5 | B | 0.75 |  |
| Palm Springs Dr. to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 18.0 | 0.0 | II | 34.0 | B | 0.76 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 361.2 | 80.4 | II | 24.8 | C | 0.55 | 0.088 galveh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to S . Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 317 | 5 | Signal | 13.2 | 4.2 | II | 16.4 | E | 0.36 |  |
| S. Wayman St. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 5 | Signal | 29.4 | 6.0 | II | 30.6 | B | 0.68 |  |
| Grant St. to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,587 | 5 | Signal | 133.8 | 73.2 | II | 13.2 | E | 0.38 |  |
| CR 427/Ronald Reagan Blva. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 5 | NA | 7.8 | 0.0 | II | 32.3 | B | 0.81 |  |
| RR Tracks to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,690 | 5 | Signal | 57.0 | 24.6 | II | 20.2 | D | 0.51 |  |
| Florida Central Pkwy. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 5 | Signal | 112.2 | 46.2 | II | 18.0 | D | 0.40 |  |
| Rangeline Rd. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 5 | Signal | 70.2 | 10.2 | II | 29.2 | B | 0.65 |  |
| Palm Springs Dr. to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 5 | Signal | 13.8 | 0.0 | II | 44.3 | A | 0.99 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 437.4 | 164.4 | II | 20.5 | D | 0.46 | $0.089 \mathrm{ga} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 18

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part B - Eastbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility } \\ & \text { Type }^{1} \\ & \hline \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ |  | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{aligned} & \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | $\begin{aligned} & \text { Roadway } \\ & \text { Class } \end{aligned}$ | Roadway Segment |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 317 | 7 | Signal | 9.0 | 1.2 | II | 24.0 | c | 0.53 |  |
| Tollgate Tr. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 7 | Signal | 26.4 | 9.0 | II | 23.2 | C | 0.52 |  |
| Palm Springs Dr. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 7 | Signal | 58.2 | 5.4 | II | 35.3 | A | 0.78 |  |
| Rangeline Rd. to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 7 | Signal | 68.4 | 14.4 | II | 29.5 | B | 0.65 |  |
| Florida Central Pkwy. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 7 | NA | 28.8 | 0.0 | 11 | 40.0 | A | 1.00 |  |
| RR Tracks to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 370 | 7 | Signal | 6.6 | 0.0 | II | 38.2 | A | 0.95 |  |
| CR 427/Ronald Reagan Blvd. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,587 | 7 | Signal | 41.4 | 0.0 | II | 42.6 | A | 0.95 |  |
| Grant St. to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 7 | Signal | 19.8 | 0.0 | II | 45.5 | A | 1.01 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 258.6 | 30.0 | II | 34.7 | B | 0.77 | $0.085 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 317 | 6 | Signal | 7.8 | 0.0 | II | 27.7 | c | 0.62 |  |
| Tollgate Tr. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 49.2 | 22.2 | II | 12.4 | F | 0.28 |  |
| Palm Springs Dr. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 6 | Signal | 60.6 | 2.4 | II | 33.9 | B | 0.75 |  |
| Rangeline Rd. to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 6 | Signal | 70.8 | 13.2 | II | 28.5 | B | 0.63 |  |
| Florida Central Pkwy. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,690 | 6 | NA | 54.6 | 8.4 | II | 21.1 | D | 0.53 |  |
| RR Tracks to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 370 | 6 | Signal | 15.0 | 5.4 | 11 | 16.8 | E | 0.42 |  |
| CR 427/Ronald Reagan Blvd. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 2,587 | 6 | Signal | 45.0 | 0.0 | II | 39.2 | A | 0.87 |  |
| Grant St. to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 20.4 | 0.0 | 11 | 44.1 | A | 0.98 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 323.4 | 51.6 | II | 27.7 | C | 0.62 | $0.089 \mathrm{ga} / \mathrm{veh}$ |

Note.
Note.

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 18

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part B - Westbound Direction Summary - After Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 317 | 7 | Signal | 18.6 | 11.4 | II | 11.6 | F | 0.26 |  |
| S. Wayman St. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 7 | Signal | 35.4 | 9.6 | II | 25.4 | C | 0.56 |  |
| Grant St. to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,587 | 7 | Signal | 123.0 | 67.2 | II | 14.3 | E | 0.41 |  |
| CR 427/Ronald Reagan Blvd. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 7 | NA | 8.4 | 0.0 | II | 30.0 | B | 0.75 |  |
| RR Tracks to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,690 | 7 | Signal | 30.0 | 0.0 | II | 38.4 | A | 0.96 |  |
| Florida Central Pkwy. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 7 | Signal | 57.0 | 1.2 | II | 35.4 | A | 0.79 |  |
| Rangeline Rd. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 7 | Signal | 53.4 | 0.0 | 11 | 38.4 | A | 0.85 |  |
| Palm Springs Dr. to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 7 | Signal | 12.0 | 0.0 | II | 51.0 | A | 1.13 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 337.8 | 89.4 | II | 26.5 | C | 0.59 | 0.087 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to S. Wayman St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 317 | 6 | Signal | 7.2 | 0.6 | II | 30.0 | в | 0.67 |  |
| S. Wayman St. to Grant St. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 45 | 1,320 | 6 | Signal | 28.8 | 5.4 | II | 31.2 | B | 0.69 |  |
| Grant St. to CR 427/Ronald Reagan Blvd. | Seminole | Arterial | Outlying Business District | 1 | 2 | 0 | 35 | 2,587 | 6 | Signal | 66.6 | 16.2 | II | 26.5 | C | 0.76 |  |
| CR 427/Ronald Reagan Blvd. to RR Tracks | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 6 | NA | 7.2 | 0.0 | II | 35.0 | B | 0.87 |  |
| RR Tracks to Florida Central Pkwy. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,690 | 6 | Signal | 37.8 | 1.8 | II | 30.5 | B | 0.76 |  |
| Florida Central Pkwy. to Rangeline Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,957 | 6 | Signal | 75.0 | 16.8 | II | 26.9 | C | 0.60 |  |
| Rangeline Rd. to Palm Springs Dr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,010 | 6 | Signal | 58.8 | 1.8 | II | 34.9 | в | 0.78 |  |
| Palm Springs Dr. to Tollgate Tr. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 898 | 6 | Signal | 16.8 | 0.0 | II | 36.4 | A | 0.81 |  |
| TOTAL |  |  |  |  |  |  | 45 | 13,147 |  |  | 298.2 | 42.6 | II | 30.1 | B | 0.67 | 0.087 galveh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## SR 434 Part B

 - AM Peak
## Before Condition

Date of Collection: 1/28/2010 Distance: 2.49 miles From: Wayman St. To: Tollgate Tr.

Start Time: 7:30 AM
End Time: 9:00 AM

EB Avg Speed: 25.4 MPH B Travel Time: 5.88 MIN EB Delay Time: 1.30 MIN WB Avg Speed: 24.8 MPH
WB Travel Time: 6.02 MIN WB Delay Time: 1.34 MIN

## SR 434 Part B <br> - AM Peak

After Condition
Date of Collection: 4/14/2010 Distance: 2.49 miles From: Wayman St. To: Tollgate Tr.
Start Time: 7:30 AM End Time: 9:00 AM

EB Avg Speed: 34.7 MPH EB Avg Speed: 34.7 MPH EB Delay Time: 0.50 MIN

WB Avg Speed: 26.5 MPH WB Travel Time: 5.63 MIN WB Delay Time: 1.49 MIN



Travel Time Study

## SR 434 Part B

Before Condition
Date of Collection: 2/3/2010 Distance: 2.49 miles From: Wayman
To: Tollgate Tr.

Start Time: 4:15 PM
End Time: 5:45 PM

EB Avg Speed: 19.1 MPH EB Travel Time: 7.82 MIN EB Delay Time: 3.10 MIN

WB Avg Speed: 20.5 MPH WB Travel Time: 7.29 MIN WB Delay Time: 2.74 MIN


## SR 434 Part B - PM Peak

## After Condition

Date of Collection: 4/14/2010 Distance: 2.49 miles From: Wayman St. To: Tollgate Tr.
Start Time: 4:15 PM End Time: 5:45 PM

EB Avg Speed: 27.7 MPH EB Avg Speed: 27.7 MPH EB Delay Time: 0.86 MIN

WB Avg Speed: 30.1 MPH WB Travel Time: 4.97 MIN WB Delay Time: 0.71 MIN

$\underbrace{(0)}_{\substack{\text { METROPLAN }}}$
Level of Services:
Travel Time Study

Table 1
SR 434 Part B : Tollgate Trail to Wayman St: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1732 | 352.8 | 78.0 | 25.4 | 0.0890 | 169.74 | 154.15 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1420 | 469.2 | 186.0 | 19.1 | 0.0900 | 185.07 | 127.80 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 984 | 361.2 | 80.4 | 24.8 | 0.0880 | 98.73 | 86.59 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1567 | 437.4 | 164.4 | 20.5 | 0.0890 | 190.39 | 139.46 |

*Traffic Volumes are obtained from the latest FDOT counts.

Table 2
SR 434 Part B : Tollgate Trail to Wayman St: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1732 | 258.6 | 30.0 | 34.7 | 0.0850 | 124.42 | 147.22 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1420 | 323.4 | 51.6 | 27.7 | 0.0890 | 127.56 | 126.38 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 984 | 337.8 | 89.4 | 26.5 | 0.0870 | 92.33 | 85.61 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1567 | 298.2 | 42.6 | 30.1 | 0.0870 | 129.80 | 136.33 |

*Traffic Volumes are obtained from the latest FDOT counts.

## SR 434 Part B : Tollgate Trail to Wayman St : Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 268.46 | 216.75 | 375.46 | 257.36 |
| Total Fuel Consumption (gallons) | 240.74 | 232.83 | 267.26 | 262.71 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 897.59$ | $\$ 2,009.47$ |
| Annual User Benefit | $\$ 269,276.83$ | $\$ 602,841.66$ |
| Total Annual User Benefit $=$ | $\$ 872,118.48$ |  |
| Total Signal Retiming Annual Cost | $\$ 11,926.92$ |  |
| User Benefit / Cost Ratio | 73.12 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## SR 434

Mitchell Hammock Rd to Palm Valley Dr

## table 19

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part C - Northbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | Stop <br> Delay <br> (sec) | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ Avg. Fuel <br> Speed Limit Consump. |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | LOS |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| McCulloch Rd to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 1,373 | 5 | Signal | 24.6 | 4.2 | 1 | 38.0 | B | 0.76 |  |
| E. Palm Valley Dr. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 5 | Signal | 55.8 | 1.2 | 1 | 47.7 | A | 0.95 |  |
| Carrigan Ave. to Chapman Rd. | Seminole | Arterial | Residential Area | 2 | 3 | 0 | 50 | 5,280 | 5 | Signal | 114.0 | 34.8 | 1 | 31.6 | C | 0.63 |  |
| Chapman Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 5 | Signal | 54.6 | 9.0 | 1 | 36.9 | B | 0.74 |  |
| Alafaya Woods Blvd. to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,323 | 5 | Signal | 80.4 | 40.2 | 11 | 19.7 | D | 0.44 |  |
| Mitchell Hammock Rd. to Hilcrest St. | Seminole | Arterial | Residential Area | 0 | 1 | 0 | 45/35 | 3,062 | 5 | Signal | 60.0 | 0.0 | 11 | 34.8 | B | 0.87 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 389.4 | 89.4 | 1 | 33.1 | C | 0.66 | $0.122 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| McCulloch Rd to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 1,373 | 4 | Signal | 19.8 | 0.0 | 1 | 47.3 | A | 0.95 |  |
| E. Palm Valley Dr. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 4 | Signal | 52.2 | 0.0 | 1 | 51.0 | A | 1.02 |  |
| Carrigan Ave. to Chapman Rd. | Seminole | Arterial | Residential Area | 2 | 3 | 0 | 50 | 5,280 | 4 | Signal | 112.8 | 36.0 | 1 | 31.9 | C | 0.64 |  |
| Chapman Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 4 | Signal | 67.2 | 16.2 | 1 | 30.0 | C | 0.60 |  |
| Alafaya Woods Blvd. to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,323 | 4 | Signal | 137.4 | 85.8 | II | 11.5 | F | 0.26 |  |
| Mitchell Hammock Rd. to Hilcrest St. | Seminole | Arterial | Residential Area | 0 | 1 | 0 | 45/35 | 3,062 | 4 | Signal | 64.8 | 0.0 | 11 | 32.2 | B | 0.81 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 454.2 | 138.0 | 1 | 28.4 | C | 0.57 | $0.123 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 19

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part C - Southbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class |  |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Average Speed | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Hilcrest St to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,062 | 5 | Signal | 102.6 | 43.8 | II | 20.4 | D | 0.45 |  |
| Mitchell Hammock Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 2,323 | 5 | Signal | 42.0 | 1.2 | 11 | 37.7 | A | 0.84 |  |
| Alafaya Woods Blvd. to Chapman Rd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 5 | Signal | 72.6 | 18.6 | 1 | 27.8 | C | 0.56 |  |
| Chapman Rd. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 5,280 | 5 | Signal | 96.6 | 11.4 | 1 | 37.3 | B | 0.75 |  |
| Carrigan Ave. to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 5 | Signal | 68.4 | 7.8 | 1 | 38.9 | B | 0.78 |  |
| E. Palm Valley Dr. to McCulloch Rd | Seminole | Arterial | Residential Area | 2 | 3 | 1 | 50 | 1,373 | 5 | Signal | 61.8 | 34.8 | 1 | 15.1 | F | 0.30 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 444.0 | 117.6 | 1 | 29.0 | C | 0.58 | $0.124 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Hilcrest St to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,062 | 4 | Signal | 205.8 | 143.4 | II | 10.1 | F | 0.23 |  |
| Mitchell Hammock Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 2,323 | 4 | Signal | 44.4 | 4.2 | 11 | 35.7 | A | 0.79 |  |
| Alafaya Woods Blvd. to Chapman Rd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 4 | Signal | 109.8 | 61.2 | 1 | 18.4 | E | 0.37 |  |
| Chapman Rd. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 5,280 | 4 | Signal | 94.8 | 15.0 | 1 | 38.0 | B | 0.76 |  |
| Carrigan Ave. to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 4 | Signal | 54.6 | 0.0 | 1 | 48.8 | A | 0.98 |  |
| E. Palm Valley Dr. to McCulloch Rd | Seminole | Arterial | Residential Area | 2 | 3 | 1 | 50 | 1,373 | 4 | Signal | 28.2 | 9.0 | 1 | 33.2 | c | 0.66 |  |
| total |  |  |  |  |  |  | 50 | 18,902 |  |  | 537.6 | 232.8 | 1 | 24.0 | D | 0.48 | $0.123 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 19

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part C - Northbound Direction Summary - After Condition

| RoadwaySegment | Jurisdiction | Facility Type ${ }^{1}$ | Area$\text { Type }^{1}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | $\begin{gathered} \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed Avg. Fuel <br> Speed Limit Consump. |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| McCulloch Rd to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 1,373 | 6 | Signal | 22.2 | 0.0 | 1 | 42.2 | A | 0.84 |  |
| E. Palm Valley Dr. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 6 | Signal | 60.6 | 7.8 | 1 | 44.0 | A | 0.88 |  |
| Carrigan Ave. to Chapman Rd. | Seminole | Arterial | Residential Area | 2 | 3 | 0 | 50 | 5,280 | 6 | Signal | 79.8 | 4.8 | 1 | 45.1 | A | 0.90 |  |
| Chapman Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 6 | Signal | 48.0 | 2.4 | 1 | 42.0 | B | 0.84 |  |
| Alafaya Woods Blvd. to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,323 | 6 | Signal | 51.0 | 4.2 | 11 | 31.1 | B | 0.69 |  |
| Mitchell Hammock Rd. to Hilcrest St. | Seminole | Arterial | Residential Area | 0 | 1 | 0 | 45/35 | 3,062 | 6 | Signal | 54.0 | 0.0 | 11 | 38.7 | A | 0.97 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 315.6 | 19.2 | 1 | 40.8 | B | 0.82 | $0.122 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| McCulloch Rd to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 1,373 | 5 | Signal | 29.4 | 2.4 | 1 | 31.8 | c | 0.64 |  |
| E. Palm Valley Dr. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 5 | Signal | 61.8 | 3.0 | 1 | 43.1 | A | 0.86 |  |
| Carrigan Ave. to Chapman Rd. | Seminole | Arterial | Residential Area | 2 | 3 | 0 | 50 | 5,280 | 5 | Signal | 96.0 | 13.2 | 1 | 37.5 | B | 0.75 |  |
| Chapman Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 5 | Signal | 65.4 | 15.6 | 1 | 30.8 | C | 0.62 |  |
| Alafaya Woods Blvd. to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 2 | 2 | 0 | 45 | 2,323 | 5 | Signal | 110.4 | 57.6 | II | 14.3 | E | 0.32 |  |
| Mitchell Hammock Rd. to Hilcrest St. | Seminole | Arterial | Residential Area | 0 | 1 | 0 | 45/35 | 3,062 | 5 | Signal | 62.4 | 0.0 | II | 33.5 | B | 0.84 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 425.4 | 91.8 | 1 | 30.3 | C | 0.61 | $0.122 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## table 19

Year 2010 METROPLAN Orlando Travel Time Study
SR 434 Part C - Southbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | Stop <br> Delay <br> (sec) | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Hilcrest St to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,062 | 7 | Signal | 127.2 | 73.2 | II | 16.4 | E | 0.36 |  |
| Mitchell Hammock Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 2,323 | 7 | Signal | 33.0 | 0.0 | 11 | 48.0 | A | 1.07 |  |
| Alafaya Woods Blvd. to Chapman Rd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 7 | Signal | 45.6 | 2.4 | 1 | 44.2 | A | 0.88 |  |
| Chapman Rd. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 5,280 | 7 | Signal | 75.6 | 0.0 | 1 | 47.6 | A | 0.95 |  |
| Carrigan Ave. to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 7 | Signal | 54.0 | 0.0 | 1 | 49.3 | A | 0.99 |  |
| E. Palm Valley Dr. to McCulloch Rd | Seminole | Arterial | Residential Area | 2 | 3 | 1 | 50 | 1,373 | 7 | Signal | 51.0 | 25.2 | 1 | 18.4 | E | 0.37 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 386.4 | 100.8 | 1 | 33.4 | C | 0.67 | 0.123 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Hilcrest St to Mitchell Hammock Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 3,062 | 4 | Signal | 151.8 | 80.4 | II | 13.8 | E | 0.31 |  |
| Mitchell Hammock Rd. to Alafaya Woods Blvd. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 2,323 | 4 | Signal | 37.2 | 0.0 | 11 | 42.6 | A | 0.95 |  |
| Alafaya Woods Blvd. to Chapman Rd. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 50 | 2,957 | 4 | Signal | 40.8 | 0.0 | 1 | 49.4 | A | 0.99 |  |
| Chapman Rd. to Carrigan Ave. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 5,280 | 4 | Signal | 74.4 | 0.0 | 1 | 48.4 | A | 0.97 |  |
| Carrigan Ave. to E. Palm Valley Dr. | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 50 | 3,907 | 4 | Signal | 61.8 | 6.0 | 1 | 43.1 | A | 0.86 |  |
| E. Palm Valley Dr. to McCulloch Rd | Seminole | Arterial | Residential Area | 2 | 3 | 1 | 50 | 1,373 | 4 | Signal | 75.0 | 43.8 | 1 | 12.5 | F | 0.25 |  |
| TOTAL |  |  |  |  |  |  | 50 | 18,902 |  |  | 441.0 | 130.2 | 1 | 29.2 | c | 0.58 | $0.123 \mathrm{ga/} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



## Level of Services:

Travel Time Study
$\rightleftharpoons A$ $\qquad$
$\longrightarrow E$ Roads City Boundary

Water
0.5 Miles


Level of Services:
Travel Time Study
MEIROPLAN



Water

SR 434 Part C : Mitchell Hammock Rd to Palm Valley Dr: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1130 | 389.4 | 89.4 | 33.1 | 0.1220 | 122.23 | 137.86 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1953 | 454.2 | 138.0 | 28.4 | 0.1230 | 246.40 | 240.22 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1669 | 444.0 | 117.6 | 29.0 | 0.1240 | 205.84 | 206.96 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1108 | 537.6 | 232.8 | 24.0 | 0.1230 | 165.46 | 136.28 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 434 Part C : Mitchell Hammock Rd to Palm Valley Dr: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1130 | 315.6 | 19.2 | 40.8 | 0.1220 | 99.06 | 137.86 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1953 | 425.4 | 91.8 | 30.3 | 0.1240 | 230.78 | 242.17 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1669 | 386.4 | 100.8 | 33.4 | 0.1230 | 179.14 | 205.29 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1108 | 441.0 | 130.2 | 29.2 | 0.1230 | 135.73 | 136.28 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 434 Part C : Mitchell Hammock Rd to Palm Valley Dr: Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 328.07 | 278.20 | 411.86 | 366.51 |
| Total Fuel Consumption (gallons) | 344.82 | 343.15 | 376.50 | 378.46 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 847.76$ | $\$ 760.69$ |
| Annual User Benefit | $\$ 254,327.92$ | $\$ 228,205.56$ |
| Total Annual User Benefit $=$ | $\$ 482,533.47$ |  |
| Total Signal Retiming Annual Cost | $\$ 9,678.71$ |  |
| User Benefit / Cost Ratio | 49.86 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## SR 436

## Wilshire Dr to Casselton Dr

## TABLE 20

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Northbound Direction Summary - Before Condition


## TABLE 20

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Southbound Direction Summary - Before Condition

| Roadway <br> Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{array}{\|c\|} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \\ \hline \end{gathered}$ | Distance <br> (ft) | \# Runs | $\begin{array}{l\|} \hline \text { Traffic } \\ \text { Control } \\ \text { Device } \\ \hline \end{array}$ | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & (\mathrm{sec}) \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \\ \hline \end{gathered}$ | Roadway Class | Roadway Segment <br> Average Speed |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fern Park Blvd. to Wilshire Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,851 | 4 | Signal | 54.0 | 4.8 | II | 36.0 | A | 0.80 |  |
| Wilshire Blvd. to Kewanee Tr. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,373 | 4 | Signal | 20.4 | 0.0 | II | 45.9 | A | 1.02 |  |
| Kewanee Tr. to Red Bug Lake Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,109 | 4 | Signal | 15.6 | 0.0 | 11 | 48.5 | A | 1.08 |  |
| Red Bug Lake Rd. to Sausalito Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 4 | Signal | 17.4 | 2.4 | 11 | 35.2 | A | 0.78 |  |
| Sausalito Blvd. to Lake Howell Rd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45/50 | 1,795 | 4 | Signal | 26.4 | 0.0 | 1 | 46.4 | A | 1.03 |  |
| Lake Howell Rd. to Lake Howell Square | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 31.8 | 6.0 | 1 | 28.3 | C | 0.57 |  |
| Lake Howell Square to Lake Howell Ln. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,373 | 4 | Signal | 19.8 | 0.0 | 1 | 47.3 | A | 0.95 |  |
| Lake Howell Ln. to Howell Branch Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,851 | 4 | Signal | 70.8 | 16.2 | 1 | 27.5 | C | 0.55 |  |
| Howell Branch Rd. to Winter Woods Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,429 | 4 | Signal | 45.0 | 1.2 | 1 | 36.8 | B | 0.74 |  |
| Winter Woods Blvd. to Castelton Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 54.6 | 7.2 | 1 | 34.3 | B | 0.69 |  |
| Castelton Blvd. to Aloma Ave | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 147.6 | 84.0 | 1 | 12.7 | F | 0.25 |  |
| TOTAL |  |  |  |  |  |  | 50 | 21,490 |  |  | 503.4 | 121.8 | 1 | 29.1 | c | 0.58 | 0.141 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fern Park Blvd. to Wilshire Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,851 | 5 | Signal | 66.6 | 13.2 | II | 29.2 | B | 0.65 |  |
| Wilshire Blvd. to Kewanee Tr. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,373 | 5 | Signal | 67.2 | 30.6 | II | 13.9 | E | 0.31 |  |
| Kewanee Tr. to Red Bug Lake Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,109 | 5 | Signal | 19.2 | 0.6 | II | 39.4 | A | 0.87 |  |
| Red Bug Lake Rd. to Sausalito Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 5 | Signal | 33.0 | 16.8 | II | 18.5 | D | 0.41 |  |
| Sausalito Blvd. to Lake Howell Rd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45/50 | 1,795 | 5 | Signal | 41.4 | 9.0 | 1 | 29.6 | C | 0.66 |  |
| Lake Howell Rd. to Lake Howell Square | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 5 | Signal | 43.8 | 13.8 | 1 | 20.5 | E | 0.41 |  |
| Lake Howell Square to Lake Howell Ln. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,373 | 5 | Signal | 21.6 | 0.0 | 1 | 43.3 | A | 0.87 |  |
| Lake Howell Ln. to Howell Branch Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,851 | 5 | Signal | 144.0 | 82.2 | 1 | 13.5 | F | 0.27 |  |
| Howell Branch Rd. to Winter Woods Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,429 | 5 | Signal | 72.6 | 22.8 | 1 | 22.8 | D | 0.46 |  |
| Winter Woods Blvd. to Castelton Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 5 | Signal | 42.6 | 0.0 | 1 | 43.9 | A | 0.88 |  |
| Castelton Blva. to Aloma Ave | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 5 | Signal | 137.4 | 82.8 | 1 | 13.6 | F | 0.27 |  |
| TOTAL |  |  |  |  |  |  | 50 | 21,490 |  |  | 689.4 | 271.8 | 1 | 21.3 | D | 0.43 | 0.145 gal/veh |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 20

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Northbound Direction Summary - After Condition


## TABLE 20

Year 2010 METROPLAN Orlando Travel Time Study
SR 436 - Southbound Direction Summary - After Condition

|  |  |  |  | Left |  | Right | Speed |  |  | Traffic | Travel | Stop |  | Roadwa | gment | Roadway | Summary |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  | Facility | Area | Turn | Thru | Turn | Limit | Distance |  | Control | Tin | Delay | Roadway | Avera | peed | Avg Speed 1 | Avg. Fuel |
| Segment | Jurisdiction | Type ${ }^{1}$ | Type ${ }^{1}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | Lanes ${ }^{2}$ | (mph) |  | \# Runs | Device | (sec) | (sec) | Class | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fern Park Blvd. to Wilshire Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,851 | 4 | Signal | 58.2 | 9.6 | II | 33.4 | B | 0.74 |  |
| Wilshire Blvd. to Kewanee Tr. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,373 | 4 | Signal | 29.4 | 6.6 | 11 | 31.8 | B | 0.71 |  |
| Kewanee Tr. to Red Bug Lake Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,109 | 4 | Signal | 43.8 | 22.8 | II | 17.3 | D | 0.38 |  |
| Red Bug Lake Rd. to Sausalito Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 4 | Signal | 13.2 | 0.0 | 11 | 46.4 | A | 1.03 |  |
| Sausalito Blvd. to Lake Howell Rd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45/50 | 1,795 | 4 | Signal | 24.6 | 0.0 | 1 | 49.8 | A | 1.11 |  |
| Lake Howell Rd. to Lake Howell Square | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 17.4 | 0.0 | 1 | 51.7 | A | 1.03 |  |
| Lake Howell Square to Lake Howell Ln. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,373 | 4 | Signal | 30.6 | 3.0 | 1 | 30.6 | C | 0.61 |  |
| Lake Howell Ln. to Howell Branch Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,851 | 4 | Signal | 54.0 | 0.6 | 1 | 36.0 | B | 0.72 |  |
| Howell Branch Rd. to Winter Woods Blva. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,429 | 4 | Signal | 45.0 | 5.4 | 1 | 36.8 | B | 0.74 |  |
| Winter Woods Blvd. to Castelton Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 49.2 | 4.8 | 1 | 38.0 | B | 0.76 |  |
| Castelton Blvd. to Aloma Ave | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 114.6 | 58.8 | 1 | 16.3 | E | 0.33 |  |
| TOTAL |  |  |  |  |  |  | 50 | 21,490 |  |  | 480.0 | 111.6 | 1 | 30.5 | C | 0.61 | 0.141 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Fern Park Blvd. to Wilshire Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 2,851 | 4 | Signal | 69.6 | 15.6 | II | 27.9 | C | 0.62 |  |
| Wilshire Blvd. to Kewanee Tr. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 1,373 | 4 | Signal | 51.6 | 23.4 | II | 18.1 | D | 0.40 |  |
| Kewanee Tr. to Red Bug Lake Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 45 | 1,109 | 4 | Signal | 54.6 | 28.2 | II | 13.8 | E | 0.31 |  |
| Red Bug Lake Rd. to Sausalito Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45 | 898 | 4 | Signal | 15.0 | 0.0 | 11 | 40.8 | A | 0.91 |  |
| Sausalito Blvd. to Lake Howell Rd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 45/50 | 1,795 | 4 | Signal | 25.2 | 0.0 | 1 | 48.6 | A | 1.08 |  |
| Lake Howell Rd. to Lake Howell Square | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,320 | 4 | Signal | 48.0 | 15.0 | 1 | 18.7 | E | 0.37 |  |
| Lake Howell Square to Lake Howell Ln. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 1,373 | 4 | Signal | 23.4 | 0.0 | 1 | 40.0 | B | 0.80 |  |
| Lake Howell Ln. to Howell Branch Rd. | Seminole | Arterial | Outlying Business District | 2 | 3 | 1 | 50 | 2,851 | 4 | Signal | 127.2 | 67.8 | 1 | 15.3 | F | 0.31 |  |
| Howell Branch Rd. to Winter Woods Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,429 | 4 | Signal | 48.0 | 3.6 | 1 | 34.5 | B | 0.69 |  |
| Winter Woods Blvd. to Castelton Blvd. | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 46.8 | 0.0 | 1 | 40.0 | B | 0.80 |  |
| Castelton Blvd. to Aloma Ave | Seminole | Arterial | Outlying Business District | 1 | 3 | 1 | 50 | 2,746 | 4 | Signal | 99.0 | 42.6 | 1 | 18.9 | E | 0.38 |  |
| total |  |  |  |  |  |  | 50 | 21,490 |  |  | 608.4 | 196.2 | 1 | 24.1 | D | 0.48 | $0.143 \mathrm{gal} / \mathrm{veh}$ |

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Mode.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.


Travel Time Study


0.5 Miles

SR 436 : Wilshire Dr to Casselton Dr : Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 2418 | 435.0 | 74.4 | 33.7 | 0.1400 | 292.18 | 338.52 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2434 | 720.6 | 276.0 | 20.3 | 0.1460 | 487.21 | 355.36 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2041 | 503.4 | 121.8 | 29.1 | 0.1410 | 285.40 | 287.78 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2721 | 689.4 | 271.8 | 21.3 | 0.1450 | 521.07 | 394.55 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 436 : Wilshire Dr to Casselton Dr : Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 2418 | 409.2 | 60.0 | 35.8 | 0.1390 | 274.85 | 336.10 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2434 | 589.2 | 211.8 | 24.9 | 0.1410 | 398.36 | 343.19 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2041 | 480.0 | 111.6 | 30.5 | 0.1410 | 272.13 | 287.78 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 2721 | 608.4 | 196.2 | 24.1 | 0.1430 | 459.85 | 389.10 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 436 : Wilshire Dr to Casselton Dr : Before \& After Study

 Summary of Measures of Effectiveness \& Benefit Cost Analysis| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 577.57 | 546.98 | $1,008.28$ | 858.21 |
| Total Fuel Consumption (gallons) | 626.30 | 623.88 | 749.91 | 732.30 |


|  | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| BENEFITS | $(7: 30-9: 00$ AM $)$ | $(4: 15-5: 45$ PM $)$ |
| User Benefit Per Day | $\$ 524.27$ | $\$ 2,588.56$ |
| Annual User Benefit | $\$ 157,280.88$ | $\$ 776,567.07$ |
| Total Annual User Benefit $=$ | $\$ 933,847.95$ |  |
| Total Signal Retiming Annual Cost | $\$ 17,509.32$ |  |
| User Benefit Cost Ratio | 53.33 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## SR 46

## Park Dr to Sanford Ave

## TABLE 21

Year 2010 METROPLAN Orlando Travel Time Study
SR 46 - Eastbound Direction Summary - Before Condition

| Roadway | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | Roadway <br> Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 17/92 to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 6 | Signal | 40.2 | 7.8 | 11 | 27.8 | c | 0.79 |  |
| Park Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 6 | Signal | 64.2 | 40.2 | II | 10.1 | F | 0.29 |  |
| Sanford Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,478 | 6 | NA Flash | 28.2 | 0.0 | 11 | 35.7 | A | 1.02 |  |
| RR Tracks to Mellonville Ave. | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 40 | 1,214 | 6 | Signal | 19.8 | 0.0 | 11 | 41.8 | A | 1.05 |  |
| TOTAL |  |  |  |  |  |  | 35 | 5,280 |  |  | 152.4 | 48.0 | II | 23.6 | C | 0.67 | $0.036 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 17/92 to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 9 | Signal | 39.0 | 6.0 | 11 | 28.6 | B | 0.82 |  |
| Park Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 9 | Signal | 49.2 | 24.6 | 11 | 13.2 | E | 0.38 |  |
| Sanford Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,478 | 9 | NA Flash | 27.6 | 0.0 | 11 | 36.5 | A | 1.04 |  |
| Sanford Ave. to Mellonville Ave. | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 40 | 1,214 | 9 | Signal | 19.8 | 0.0 | II | 41.8 | A | 1.05 |  |
| TOTAL |  |  |  |  |  |  | 35 | 5,280 |  |  | 135.6 | 30.6 | II | 26.5 | c | 0.76 | $0.035 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type <br> 2. The Through lanes and Turn la | ed from the lat | t Orlando | an Area Transpor | n Study | UATS) M |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 21

Year 2010 METROPLAN Orlando Travel Time Study
SR 46 - Westbound Direction Summary - Before Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | Thru Lanes ${ }^{2}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \\ \hline \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | $\begin{gathered} \hline \text { Travel } \\ \text { Time } \\ \text { (sec) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed $/$ Avg. Fuel <br> Speed Limit Consump. |  |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Mellonville Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 40 | 1,214 | 6 | NA | 19.8 | 0.0 | II | 41.8 | A | 1.05 |  |
| RR Tracks to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,478 | 6 | Signal | 38.4 | 11.4 | II | 26.2 | C | 0.66 |  |
| Sanford Ave. to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 6 | Signal | 18.6 | 0.0 | II | 34.8 | B | 1.00 |  |
| Park Ave. to US 17/92 | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 35 | 1,637 | 6 | Signal | 64.8 | 28.8 | II | 17.2 | D | 0.49 |  |
| TOTAL |  |  |  |  |  |  | 40 | 5,280 |  |  | 141.6 | 40.2 | II | 25.4 | C | 0.64 | 0.035 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Mellonville Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 40 | 1,214 | 9 | NA | 19.8 | 0.0 | II | 41.8 | A | 1.05 |  |
| RR Tracks to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,478 | 9 | Signal | 56.4 | 24.6 | II | 17.9 | D | 0.45 |  |
| Sanford Ave. to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 9 | Signal | 18.6 | 0.0 | II | 34.8 | B | 1.00 |  |
| Park Ave. to US 17/92 | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 35 | 1,637 | 9 | Signal | 80.4 | 45.0 | II | 13.9 | E | 0.40 |  |
| TOTAL |  |  |  |  |  |  | 40 | 5,280 |  |  | 175.2 | 69.6 | II | 20.5 | D | 0.51 | $0.036 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 21

Year 2010 METROPLAN Orlando Travel Time Study
SR 46 - Eastbound Direction Summary - After Condition

| Roadway | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | Roadway <br> Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 17/92 to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 12 | Signal | 32.4 | 1.2 | 11 | 34.4 | в | 0.98 |  |
| Park Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 12 | Signal | 40.2 | 17.4 | II | 16.1 | E | 0.46 |  |
| Sanford Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,478 | 12 | NA Flash | 25.8 | 0.0 | 11 | 39.1 | A | 1.12 |  |
| RR Tracks to Mellonville Ave. | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 40 | 1,214 | 12 | Signal | 18.6 | 0.0 | 11 | 44.5 | A | 1.11 |  |
| TOTAL |  |  |  |  |  |  | 35 | 5,280 |  |  | 117.0 | 18.6 | II | 30.8 | B | 0.88 | 0.035 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 17/92 to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 1,637 | 11 | Signal | 33.6 | 2.4 | 11 | 33.2 | B | 0.95 |  |
| Park Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 11 | Signal | 43.2 | 21.0 | 11 | 15.0 | E | 0.43 |  |
| Sanford Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 35 | 1,478 | 11 | NA Flash | 25.2 | 0.0 | 11 | 40.0 | A | 1.14 |  |
| Sanford Ave. to Mellonville Ave. | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 40 | 1,214 | 11 | Signal | 24.0 | 0.0 | II | 34.5 | B | 0.86 |  |
| TOTAL |  |  |  |  |  |  | 35 | 5,280 |  |  | 126.0 | 23.4 | II | 28.6 | B | 0.82 | 0.035 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type | ed from the lat | t Orlando | an Area Transpor | n Study | UATS) M |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 21

Year 2010 METROPLAN Orlando Travel Time Study
SR 46 - Westbound Direction Summary - After Condition

| Roadway <br> Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Type }^{1}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | Speed Limit (mph) | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Mellonville Ave. to RR Tracks | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 40 | 1,214 | 12 | NA | 19.8 | 0.0 | II | 41.8 | A | 1.05 |  |
| RR Tracks to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,478 | 12 | Signal | 39.0 | 13.2 | II | 25.8 | C | 0.65 |  |
| Sanford Ave. to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 12 | Signal | 19.2 | 1.8 | II | 33.7 | B | 0.96 |  |
| Park Ave. to US 17/92 | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 35 | 1,637 | 12 | Signal | 36.0 | 4.8 | II | 31.0 | B | 0.89 |  |
| TOTAL |  |  |  |  |  |  | 40 | 5,280 |  |  | 114.0 | 19.8 | II | 31.6 | B | 0.79 | $0.035 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CON |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Mellonville Ave. to Sanford Ave. | Seminole | Arterial | Residential Area | 0 | 2 | 0 | 40 | 1,214 | 11 | NA | 20.4 | 0.0 | II | 40.6 | A | 1.01 |  |
| RR Tracks to Sanford Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 1,478 | 11 | Signal | 28.2 | 0.6 | II | 35.7 | A | 0.89 |  |
| Sanford Ave. to Park Ave. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 35 | 950 | 11 | Signal | 17.4 | 0.0 | II | 37.2 | A | 1.06 |  |
| Park Ave. to US $17 / 92$ | Seminole | Arterial | Residential Area | 1 | 1 | 1 | 35 | 1,637 | 11 | Signal | 61.2 | 25.2 | II | 18.2 | D | 0.52 |  |
| TOTAL |  |  |  |  |  |  | 40 | 5,280 |  |  | 127.2 | 25.8 | II | 28.3 | B | 0.71 | $0.036 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.


Before Condition
Date of Collection: 2/4/2010 Distance: 1.00 mile From: Mellonville Ave. To: US 17/92

Start Time: 4:30 PM End Time: 6:00 PM

EB Avg Speed: 26.5 MPH EB Travel Time: 2.26 MIN EB Delay Time: 0.51 MIN

WB Avg Speed: 20.5 MPH WB Travel Time: 2.92 MIN WB Delay Time: 1.16 MIN


SR 46 - PM Peak

After Condition
Date of Collection: 5/12/2010 Distance: 1.00 mile From: Mellonville Ave
To: US 17/92
Start Time: 4:30 PM End Time: 6:00 PM

EB Avg Speed: 28.6 MPH EB Avg Speed: 28.6 MPH
EB Travel Time: 2.10 MIN EB Delay Time: 0.39 MIN

WB Avg Speed: 28.3 MPH WB Travel Time: 2.12 MIN WB Delay Time: 0.43 MIN
 Level of Services:

SR 46 : Park Dr to Sanford Ave : Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 568 | 152.4 | 48.0 | 23.6 | 0.0360 | 24.05 | 20.45 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 941 | 135.6 | 30.6 | 26.5 | 0.0350 | 35.44 | 32.94 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 841 | 141.6 | 40.2 | 25.4 | 0.0350 | 33.08 | 29.44 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 612 | 175.2 | 69.6 | 20.5 | 0.0360 | 29.78 | 22.03 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 46 : Park Dr to Sanford Ave : Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 568 | 117.0 | 18.6 | 30.8 | 0.0350 | 18.46 | 19.88 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 941 | 126.0 | 23.4 | 28.6 | 0.0350 | 32.94 | 32.94 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 841 | 114.0 | 19.8 | 31.6 | 0.0350 | 26.63 | 29.44 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 612 | 127.2 | 25.8 | 28.3 | 0.0360 | 21.62 | 22.03 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 46 : Park Dr to Sanford Ave : Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 57.12 | 45.09 | 65.23 | 54.56 |
| Total Fuel Consumption (gallons) | 49.88 | 49.32 | 54.97 | 54.97 |


| BENEFITS | AM PEAK HOUR <br> $(7: 30-9: 00 ~ A M)$ | PM PEAK HOUR <br> $(4: 15-5: 45$ PM $)$ |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 205.05$ | $\$ 180.31$ |
| Annual User Benefit | $\$ 61,515.10$ | $\$ 54,093.52$ |
| Total Annual User Benefit $=$ | $\$ 115,608.62$ |  |
| Total Signal Retiming Annual Cost | $\$ 3,905.78$ |  |
| User Benefit Cost Ratio | 29.60 |  |

## Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

CR 46A

## SR 417 NB Ramp to SR 417 SB Ramp

## TABLE 22

Year 2010 METROPLAN Orlando Travel Time Study
CR 46A - Eastbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Typef }^{2}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c} \text { Thru } \\ \text { Lanes }^{2} \end{array}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time $\qquad$ <br> (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Casa Verde Blvd. to SR 417 SB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 898 | 8 | Signal | 14.4 | 0.0 | II | 42.5 | A | 1.06 |  |
| SR 417 SB to SR 417 NB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 8 | Signal | 12.0 | 3.0 | II | 33.0 | B | 0.82 |  |
| SR 417 NB to Airport Blvd. | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 40 | 1,056 | 8 | Signal | 51.6 | 27.6 | II | 14.0 | E | 0.35 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 78.0 | 30.6 | II | 22.2 | C | 0.55 | 0.017 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Casa Verde Blvd. to SR 417 SB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 898 | 7 | Signal | 17.4 | 0.6 | II | 35.2 | A | 0.88 |  |
| SR 417 SB to SR 417 NB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 7 | Signal | 10.8 | 0.0 | " | 36.7 | A | 0.92 |  |
| SR 417 NB to Airport Blvd. | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 40 | 1,056 | 7 | Signal | 45.6 | 19.2 | II | 15.8 | E | 0.39 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 73.8 | 19.8 | II | 23.4 | c | 0.59 | 0.017 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 22

Year 2010 METROPLAN Orlando Travel Time Study
CR 46A - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Typef }^{2}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c} \text { Thru } \\ \text { Lanes }^{2} \end{array}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{gathered} \hline \text { Travel } \\ \text { Time } \\ \text { (sec) } \end{gathered}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Airport Blvd. to SR 417 NB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 1,056 | 8 | Signal | 18.6 | 0.0 | II | 38.7 | A | 0.97 |  |
| SR 417 NB to SR 417 SB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 8 | Signal | 9.0 | 0.0 | II | 44.0 | A | 1.10 |  |
| SR 417 SB to Casa Verde Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 898 | 8 | Signal | 13.8 | 0.0 | II | 44.3 | A | 1.11 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 41.4 | 0.0 | II | 41.7 | A | 1.04 | 0.016 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Airport Blva. to SR 417 NB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 1,056 | 7 | Signal | 39.0 | 13.8 | II | 18.5 | D | 0.46 |  |
| SR 417 NB to SR 417 SB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 7 | Signal | 11.4 | 0.0 | " | 34.7 | B | 0.87 |  |
| SR 417 SB to Casa Verde Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 898 | 7 | Signal | 13.2 | 0.0 | II | 46.4 | A | 1.16 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 63.6 | 13.8 | II | 27.2 | C | 0.68 | 0.017 gal/veh |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TABLE 22

Year 2010 METROPLAN Orlando Travel Time Study
CR 46A - Eastbound Direction Summary - After Condition


## TABLE 22

Year 2010 METROPLAN Orlando Travel Time Study
CR 46A - Westbound Direction Summary - After Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility }^{\text {Typef }^{2}} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c} \text { Thru } \\ \text { Lanes }^{2} \end{array}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{aligned} & \hline \text { Speed } \\ & \text { Limit } \\ & (\mathrm{mph}) \\ & \hline \end{aligned}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{gathered} \hline \text { Travel } \\ \text { Time } \\ \text { (sec) } \end{gathered}$ | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ (\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { Roadway } \\ \text { Class } \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ | Avg. Fuel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los | Speed Limit | Consump. |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Airport Blvd. to SR 417 NB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 1,056 | 12 | Signal | 17.4 | 0.0 | II | 41.4 | A | 1.03 |  |
| SR 417 NB to SR 417 SB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 12 | Signal | 9.2 | 0.0 | II | 43.0 | A | 1.08 |  |
| SR 417 SB to Casa Verde Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 898 | 12 | Signal | 13.8 | 0.0 | II | 44.3 | A | 1.11 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 40.4 | 0.0 | II | 42.8 | A | 1.07 | 0.016 gal/veh |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Airport Blva. to SR 417 NB | Seminole | Arterial | Residential Area | 0 | 2 | 1 | 40 | 1,056 | 16 | Signal | 19.2 | 0.0 | 11 | 37.5 | A | 0.94 |  |
| SR 417 NB to SR 417 SB | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 581 | 16 | Signal | 13.2 | 3.0 | " | 30.0 | в | 0.75 |  |
| SR 417 SB to Casa Verde Blvd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 40 | 898 | 16 | Signal | 17.4 | 2.4 | II | 35.2 | A | 0.88 |  |
| TOTAL |  |  |  |  |  |  | 40 | 2,534 |  |  | 49.8 | 5.4 | II | 34.7 | B | 0.87 | $0.016 \mathrm{gal} / \mathrm{veh}$ |
| Note: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model. <br> 2. The Through lanes and Turn lanes are provided for the approach of the direction of travel. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |




CR 46 A : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 764 | 78.0 | 30.6 | 22.2 | 0.0170 | 16.55 | 12.99 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1229 | 73.8 | 19.8 | 23.4 | 0.0170 | 25.19 | 20.89 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 995 | 41.4 | 0.0 | 41.7 | 0.0160 | 11.44 | 15.92 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 827 | 63.6 | 13.8 | 27.2 | 0.0170 | 14.61 | 14.06 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

CR 46 A : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 764 | 60.6 | 15.0 | 28.5 | 0.0170 | 12.86 | 12.99 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1229 | 66.0 | 12.0 | 26.2 | 0.0170 | 22.53 | 20.89 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 995 | 40.4 | 0.0 | 42.8 | 0.0160 | 11.17 | 15.92 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 827 | 49.8 | 5.4 | 34.7 | 0.0160 | 11.44 | 13.23 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

CR 46 A : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 28.00 | 24.03 | 39.80 | 33.97 |
| Total Fuel Consumption (gallons) | 28.91 | 28.91 | 34.95 | 34.13 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 67.08$ | $\$ 101.04$ |
| Annual User Benefit | $\$ 20,123.11$ | $\$ 30,312.65$ |
| Total Annual User Benefit $=$ | $\$ 50,435.76$ |  |
| Total Signal Retiming Annual Cost | $\$ 3,861.20$ |  |
| User Benefit Cost Ratio | $\mathbf{1 3 . 0 6}$ |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.

SR 426

## Old Howell Branch Rd to Dean Rd

## TABLE 23

Year 2010 METROPLAN Orlando Travel Time Study
SR 426 - Eastbound Direction Summary - Before Condition


Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 23

Year 2010 METROPLAN Orlando Travel Time Study
SR 426 - Westbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ |  | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | Right Turn Lanes ${ }^{2}$ | Speed <br> Limit <br> (mph) | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | Travel Time (sec) | Stop <br> Delay <br> (sec) | Roadway <br> Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel <br> Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dean Rd. | Seminole | Arterial | Rural Area | 0 | 2 | 1 | 45 | 686 | 8 | Signal | 48.6 | 30.6 | 11 | 9.6 | F | 0.21 |  |
| Dean Rd. to SR 417 (E. Ramps) | Seminole | Arterial | Rural Area | 0 | 2 | 1 | 45 | 2,851 | 8 | Signal | 55.8 | 3.6 | II | 34.8 | B | 0.77 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 8 | Signal | 11.4 | 2.4 | II | 22.1 | C | 0.49 |  |
| SR 417 (W. Ramps) to Deep Lake St. | Seminole | Arterial | Residential Area | 1 | 2 | 1 | 45 | 845 | 8 | Signal | 24.0 | 3.6 | II | 24.0 | C | 0.53 |  |
| Deep Lake St.Tuskawilla Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 8 | Signal | 35.4 | 8.4 | II | 26.4 | C | 0.59 |  |
| Tuskawilla Rd. to Trinity Prep. Sch. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 8 | Signal | 42.6 | 0.0 | II | 43.1 | A | 0.96 |  |
| Trinity Prep. Sch. to Hall Rd./Howell Branch Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,112 | 8 | Signal | 68.4 | 25.2 | 11 | 21.1 | D | 0.47 |  |
| Hall Rd./Howell Branch Rd. to Old Howell Branch Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,214 | 8 | Signal | 21.6 | 0.0 | II | 38.3 | A | 0.85 |  |
| Old Howell Branch Rd. to Median Opening | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 475 | 8 | Signal | 6.6 | 0.0 | II | 49.1 | A | 1.09 |  |
| TOTAL |  |  |  |  |  |  | 45 | 12,619 |  |  | 314.4 | 73.8 | II | 27.4 | C | 0.61 | 0.085 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Opening to Dean Rd. | Seminole | Arterial | Rural Area | 0 | 2 | 1 | 45 | 686 | 8 | Signal | 57.6 | 40.8 | II | 8.1 | F | 0.18 |  |
| Dean Rd. to SR 417 (E. Ramps) | Seminole | Arterial | Rural Area | 0 | 2 | 1 | 45 | 2,851 | 8 | Signal | 57.6 | 4.8 | 11 | 33.7 | B | 0.75 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 370 | 8 | Signal | 6.6 | 0.0 | II | 38.2 | A | 0.85 |  |
| SR 417 (W. Ramps) to Deep Lake St. | Seminole | Arterial | Residential Area | 1 | 2 | 1 | 45 | 845 | 8 | Signal | 15.0 | 0.0 | II | 38.4 | A | 0.85 |  |
| Deep Lake St.Tuskawilla Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 1 | 45 | 1,373 | 8 | Signal | 33.0 | 3.0 | II | 28.4 | B | 0.63 |  |
| Tuskawilla Rd. to Trinity Prep. Sch. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,693 | 8 | Signal | 43.8 | 0.0 | 11 | 41.9 | A | 0.93 |  |
| Trinity Prep. Sch. to Hall Rd./Howell Branch Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 2,112 | 8 | Signal | 45.6 | 9.0 | 11 | 31.6 | B | 0.70 |  |
| Hall Rd./Howell Branch Rd. to Old Howell Branch Rd. | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 1,214 | 8 | Signal | 19.8 | 0.0 | II | 41.8 | A | 0.93 |  |
| Old Howell Branch Rd. to Median Opening | Seminole | Arterial | Residential Area | 1 | 2 | 0 | 45 | 475 | 8 | Signal | 6.6 | 0.0 | II | 49.1 | A | 1.09 |  |
| total |  |  |  |  |  |  | 45 | 12,619 |  |  | 285.6 | 57.6 | 1 | 30.1 | B | 0.67 | $0.083 \mathrm{gal} / \mathrm{veh}$ |

## Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 23

Year 2010 METROPLAN Orlando Travel Time Study SR 426 - Eastbound Direction Summary - After Condition


## Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 23

Year 2010 METROPLAN Orlando Travel Time Study
SR 426 - Westbound Direction Summary - After Condition


1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

SR 426 - AM Peak

## Before Condition

Date of Collection: 1/28/2010 Distance: 2.39 miles From: Dean Rd. To: Old Howell Branch Rd.

Start Time: 7:15 AM End Time: 8:45 AM

EB Avg Speed: 32.2 MPH EB Travel Time: 4.45 MIN EB Delay Time: 0.79 MIN

WB Avg Speed: 27.4 MPH WB Travel Time: 5.24 MIN WB Delay Time: 1.23 MIN

## SR 426

- AM Peak

After Condition
Date of Collection: 5/6/2010 Distance: 2.39 miles
From: Dean Rd
To: Old Howell Branch Rd.
Start Time: 7:15 AM Start Time: $7: 15 \mathrm{AM}$
End Time: $8: 45 \mathrm{AM}$
EB Avg Speed: 34.0 MPH EB Travel Time: 4.22 MIN EB Delay Time: 0.62 MIN

WB Avg Speed: 35.5 MPH WB Travel Time: 4.04 MIN WB Delay Time: 0.79 MIN


2010 MEIROPLAN ORLANDO
Travel Time Study
$0.5 \quad$ Miles

SR 426 - PM Peak

## Before Condition

Date of Collection: 1/28/2010 Distance: 2.39 miles From: Dean Rd. To: Old Howell Branch Rd.

Start Time: 4:30 PM End Time: 6:00 PM

EB Avg Speed: 26.8 MPH B Travel Time: 5.36 MIN EB Delay Time: 1.36 MIN

WB Avg Speed: 30.1 MPH WB Travel Time: 4.76 MIN WB Delay Time: 0.96 MIN

## SR 426

- PM Peak

After Condition
Date of Collection: 3/31/2010 Distance: 2.39 miles
to: Old Howell Branch Rd.
Start Time: 4:30 PM Start Time: 4:30 PM
End Time: 6:00 PM
EB Avg Speed: 29.8 MPH EB Travel Time: 4.82 MIN EB Delay Time: 0.87 MIN

WB Avg Speed: 31.5 MPH WB Travel Time: 4.55 MIN WB Delay Time: 0.75 MIN



SR 426 : Old Howell Branch Rd to Dean Rd : Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1759 | 267.0 | 47.4 | 32.2 | 0.0820 | 130.46 | 144.24 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2076 | 321.6 | 81.6 | 26.8 | 0.0840 | 185.46 | 174.38 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2090 | 314.4 | 73.8 | 27.4 | 0.0850 | 182.53 | 177.65 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1792 | 285.6 | 57.6 | 30.1 | 0.0830 | 142.17 | 148.74 |

*Traffic Volumes are obtained from the latest FDOT Counts

SR 426 : Old Howell Branch Rd to Dean Rd : Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1759 | 253.2 | 37.2 | 34.0 | 0.0830 | 123.72 | 146.00 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 2076 | 289.2 | 52.2 | 29.8 | 0.0830 | 166.77 | 172.31 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 2090 | 242.4 | 47.4 | 35.5 | 0.0810 | 140.73 | 169.29 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1792 | 273.0 | 45.0 | 31.5 | 0.0830 | 135.89 | 148.74 |

*Traffic Volumes are obtained from the latest FDOT Counts

## SR 426 : Old Howell Branch Rd to Dean Rd : Before \& After Study

Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 312.99 | 264.44 | 327.62 | 302.67 |
| Total Fuel Consumption (gallons) | 321.89 | 315.29 | 323.12 | 321.04 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 840.04$ | $\$ 427.94$ |
| Annual User Benefit | $\$ 252,013.46$ | $\$ 128,382.86$ |
| Total Annual User Benefit $=$ | $\$ 380,396.32$ |  |
| Total Signal Retiming Annual Cost | $\$ 13,717.86$ |  |
| User Benefit/ Cost Ratio 27.73 |  |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Red Bug Lake Rd

## SR 417 EB to SR 417 WB

## TABLE 24

Year 2010 METROPLAN Orlando Travel Time Study
Red Bug Lake Rd - Eastbound Direction Summary - Before Condition

| Roadway Segment | Jurisdiction | $\begin{aligned} & \text { Facility } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{array}{\|c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Oviedo Market Pl. to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,373 | 10 | Signal | 28.8 | 4.8 | II | 32.5 | B | 0.72 |  |
| SR 417 (W. Ramps) to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 634 | 10 | Signal | 12.6 | 0.6 | II | 34.3 | B | 0.76 |  |
| SR 417 (E. Ramps) to SR 426 | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,690 | 10 | Signal | 57.0 | 22.2 | II | 20.2 | D | 0.45 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 98.4 | 27.6 | II | 25.6 | C | 0.57 | 0.025 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Oviedo Market Pl. to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,373 | 8 | Signal | 27.6 | 3.0 | II | 33.9 | B | 0.75 |  |
| SR 417 (W. Ramps) to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 634 | 8 | Signal | 10.2 | 0.0 | II | 42.4 | A | 0.94 |  |
| SR 417 (E. Ramps) to SR 426 | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,690 | 8 | Signal | 43.8 | 13.2 | II | 26.3 | C | 0.58 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 81.6 | 16.2 | II | 30.9 | B | 0.69 | $0.024 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 24

Year 2010 METROPLAN Orlando Travel Time Study
Red Bug Lake Rd - Westbound Direction Summary - Before Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \\ \hline \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| SR 426 to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,690 | 10 | Signal | 45.6 | 13.2 | II | 25.3 | c | 0.56 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 634 | 10 | Signal | 11.4 | 0.0 | 11 | 37.9 | A | 0.84 |  |
| SR 417 ( W. Ramps) tp Oviedo Market PI. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 45 | 1,373 | 10 | Signal | 22.8 | 2.4 | II | 41.1 | A | 0.91 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 79.8 | 15.6 | II | 31.6 | B | 0.70 | 0.024 gal/veh |
| PM PEAK HOUR - BEFORE CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| SR 426 to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,690 | 8 | Signal | 41.4 | 10.8 | 11 | 27.8 | c | 0.62 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 634 | 8 | Signal | 24.6 | 9.6 | II | 17.6 | D | 0.39 |  |
| SR 417 ( (W. Ramps) tp Oviedo Market PI. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 45 | 1,373 | 8 | Signal | 40.2 | 11.4 | II | 23.3 | c | 0.52 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 106.2 | 31.8 | II | 23.7 | c | 0.53 | $0.025 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 24

Year 2010 METROPLAN Orlando Travel Time Study
Red Bug Lake Rd - Eastbound Direction Summary - After Condition

|  | Jurisdiction | Facility Type ${ }^{1}$ | Area Type ${ }^{1}$ | $\begin{gathered} \hline \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Right } \\ \text { Turn } \\ \text { Lanes }^{2} \\ \hline \end{array}$ | $\begin{gathered} \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic Control Device | Travel <br> Time <br> (sec) | $\begin{gathered} \hline \text { Stop } \\ \text { Delay } \\ \text { (sec) } \end{gathered}$ | Roadway Class | $\begin{gathered} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{gathered}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Oviedo Market Pl. to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,373 | 12 | Signal | 27.6 | 3.0 | II | 33.9 | B | 0.75 |  |
| SR 417 (W. Ramps) to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 634 | 12 | Signal | 10.8 | 0.0 | 11 | 40.0 | A | 0.89 |  |
| SR 417 (E. Ramps) to SR 426 | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,690 | 12 | Signal | 40.2 | 10.8 | 11 | 28.7 | B | 0.64 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 78.6 | 13.8 | II | 32.1 | B | 0.71 | $0.024 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Oviedo Market Pl. to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,373 | 8 | Signal | 26.4 | 1.2 | II | 35.5 | A | 0.79 |  |
| SR 417 (W. Ramps) to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 1 | 3 | 0 | 45 | 634 | 8 | Signal | 12.6 | 1.8 | 11 | 34.3 | B | 0.76 |  |
| SR 417 (E. Ramps) to SR 426 | Seminole | Arterial | Residential Area | 2 | 2 | 1 | 45 | 1,690 | 8 | Signal | 39.0 | 4.8 | II | 29.5 | B | 0.66 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 78.0 | 7.8 | II | 32.3 | B | 0.72 | $0.025 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.

## TABLE 24

Year 2010 METROPLAN Orlando Travel Time Study
Red Bug Lake Rd - Westbound Direction Summary - After Condition

|  | Jurisdiction | $\begin{aligned} & \text { Facility } \\ & \text { Type }^{1} \end{aligned}$ | $\begin{gathered} \text { Area } \\ \text { Type }^{1} \end{gathered}$ | $\begin{gathered} \text { Left } \\ \text { Turn } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{gathered} \text { Thru } \\ \text { Lanes }^{2} \end{gathered}$ | $\begin{aligned} & \hline \text { Right } \\ & \text { Turn } \\ & \text { Lanes }^{2} \end{aligned}$ | $\begin{gathered} \hline \text { Speed } \\ \text { Limit } \\ (\mathrm{mph}) \end{gathered}$ | Distance <br> (ft) | \# Runs | Traffic <br> Control <br> Device | $\begin{aligned} & \hline \text { Travel } \\ & \text { Time } \\ & \text { (sec) } \end{aligned}$ | $\begin{aligned} & \hline \text { Stop } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Roadway Class | $\begin{array}{\|c\|} \hline \text { Roadway Segment } \\ \hline \text { Average Speed } \\ \hline \end{array}$ |  | Roadway Summary |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Avg Speed/ Speed Limit | Avg. Fuel Consump. |
| Segment |  |  |  |  |  |  |  |  |  |  |  |  |  | (mph) | Los |  |  |
| AM PEAK HOUR -AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| SR 426 to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,690 | 11 | Signal | 30.0 | 0.0 | II | 38.4 | A | 0.85 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 634 | 11 | Signal | 9.6 | 0.0 | 11 | 45.0 | A | 1.00 |  |
| SR 417 (W. Ramps) tp Oviedo Market PI. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 45 | 1,373 | 11 | Signal | 24.6 | 1.8 | II | 38.0 | A | 0.85 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 64.2 | 1.8 | II | 39.3 | A | 0.87 | $0.024 \mathrm{gal} / \mathrm{veh}$ |
| PM PEAK HOUR - AFTER CONDITION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| SR 426 to SR 417 (E. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 1,690 | 9 | Signal | 25.8 | 0.0 | II | 44.6 | A | 0.99 |  |
| SR 417 (E. Ramps) to SR 417 (W. Ramps) | Seminole | Arterial | Residential Area | 0 | 3 | 1 | 45 | 634 | 9 | Signal | 9.0 | 0.0 | II | 48.0 | A | 1.07 |  |
| SR 417 (W. Ramps) tp Oviedo Market PI. | Seminole | Arterial | Residential Area | 1 | 3 | 1 | 45 | 1,373 | 9 | Signal | 39.6 | 15.6 | II | 23.6 | c | 0.53 |  |
| TOTAL |  |  |  |  |  |  | 45 | 3,696 |  |  | 74.4 | 15.6 | II | 33.9 | B | 0.75 | $0.024 \mathrm{gal} / \mathrm{veh}$ |

Note:

1. The Facility type and Area type definitions were obtained from the latest Orlando Urban Area Transportation Study (OUATS) Model.
2. The Through lanes and Turn lanes are provided for the approach of the direction of travel.



2010 MEIROPLAN ORLANDO
Travel Time Study


2010 MEIROPLAN ORLANDO
Travel Time Study

Red Bug Lake Rd : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study
Summary of Before Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1383 | 98.4 | 27.6 | 25.6 | 0.0250 | 37.80 | 34.58 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1987 | 81.6 | 16.2 | 30.9 | 0.0240 | 45.04 | 47.69 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1907 | 79.8 | 15.6 | 31.6 | 0.0240 | 42.27 | 45.77 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1535 | 106.2 | 31.8 | 23.7 | 0.0250 | 45.28 | 38.38 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

Red Bug Lake Rd : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study
Summary of After Study Travel Time and Delay Study Results

|  | MOE's PER VEHICLE |  |  |  | MOE's FOR ALL THE VEHICLES PASSING THROUGH THE ROADWAY SEGMENT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Traffic Volume | Travel Time (sec/veh) | Delay (sec/veh) | Average Speed (mph) | Fuel Consumption (gallons/veh) | Total Travel Time (Veh-hour) | Total Fuel Consumption (in gallons) |
| Northbound/Eastbound - AM Peak Hour |  |  |  |  |  |  |
| 1383 | 78.6 | 13.8 | 32.1 | 0.0240 | 30.20 | 33.19 |
| Northbound/Eastbound - PM Peak Hour |  |  |  |  |  |  |
| 1987 | 78.0 | 7.8 | 32.3 | 0.0250 | 43.05 | 49.68 |
| Southbound/Westbound - AM Peak Hour |  |  |  |  |  |  |
| 1907 | 64.2 | 1.8 | 39.3 | 0.0240 | 34.01 | 45.77 |
| Southbound/Westbound - PM Peak Hour |  |  |  |  |  |  |
| 1535 | 74.4 | 15.6 | 33.9 | 0.0240 | 31.72 | 36.84 |

*Traffic Volumes are obtained from the 2009 Seminole County Counts

Red Bug Lake Rd : SR 417 NB Ramp to SR 417 SB Ramp: Before \& After Study Summary of Measures of Effectiveness \& Benefit Cost Analysis

| MOE's | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Before | After | Before | After |
| Total Travel Time (vehicle - hrs) | 80.07 | 64.20 | 90.32 | 74.78 |
| Total Fuel Consumption (gallons) | 80.34 | 78.96 | 86.06 | 86.52 |


| BENEFITS | AM PEAK HOUR | PM PEAK HOUR |
| :---: | :---: | :---: |
| User Benefit Per Day | $\$ 272.33$ | $\$ 261.38$ |
| Annual User Benefit | $\$ 81,698.15$ | $\$ 78,414.98$ |
| Total Annual User Benefit $=$ | $\$ 160,113.12$ |  |
| Total Signal Retiming Annual Cost | $\$ 3,861.20$ |  |
| User Benefit / Cost Ratio | 41.47 |  |

Notes:

* Value of Delay Time is $\$ 16.90$ per hour ( $\$ 15.47$ from Mobility Data for Orlando for the year 2007 \& grown 3\% per year)
* Fuel consumption is valued to the rate of $\$ 2.98$ per gallon.
* Benefits apply for 300 days per year. This accounts for reduced benefits anticipated from lower weekend traffics
* The service life of the improvement was kept as three (3) years.
* Interest rate of $7 \%$ used by FDOT was used in arriving at the annual cost of improvements.


## Appendix B

Page from 2007 Urban Mobility Report

The Mobility Data for Orlando FL

| Inventory Measures | 2007 | 2006 | 2005 | 2004 | 2003 | 2002 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Urban Area Information |  |  |  |  |  |  |
| Population (1000s) | 1,405 | 1,375 | 1,360 | 1,320 | 1,290 | 1,260 |
| Rank | 32 | 33 | 33 | 33 | 33 | 33 |
| Urban Area (square miles) | 725 | 720 | 715 | 715 | 680 | 680 |
| Population Density (persons/sq mile) | 1,938 | 1,910 | 1,902 | 1,846 | 1,897 | 1,853 |
| Peak Travelers (1000s) | 787 | 765 | 751 | 725 | 704 | 678 |
| Freeway |  |  |  |  |  |  |
| Daily Vehicle-Miles of Travel (1000s) | 13,540 | 12,980 | 12,470 | 11,765 | 10,570 | 10,000 |
| Lane-Miles | 870 | 860 | 850 | 840 | 805 | 775 |
| Arterial Streets |  |  |  |  |  |  |
| Daily Vehicle-Miles of Travel (1000s) | 17,000 | 16,595 | 16,770 | 16,530 | 17,000 | 17,000 |
| Lane-Miles | 2,240 | 2,140 | 2,100 | 2,075 | 2,060 | 2,060 |
| Public Transportation |  |  |  |  |  |  |
| Annual Psgr-Miles of Travel (millions) | 159 | 163 | 160 | 144 | 147 | 144 |
| Annual Unlinked Psgr Trips (millions) | 26 | 25 | 25 | 23 | 23 | 22 |
| Cost Components |  |  |  |  |  |  |
| Value of Time (\$/hour) | 15.47 | 15.06 | 14.58 | 14.10 | 13.73 | 13.43 |
| Commercial Cost (\$/hour) | 102.12 | 98.77 | 94.06 | 86.24 | 82.38 | 79.96 |
| Fuel Cost (\$/gallon) | 2.98 | 2.66 | 2.34 | 1.99 | 1.53 | 1.41 |
| System Performance | 2007 | 2006 | 2005 | 2004 | 2003 | 2002 |
| Congested Travel (\% of peak VMT) | 74 | 72 | 70 | 68 | 69 | 71 |
| Congested System (\% of lane-miles) | 69 | 68 | 66 | 65 | 65 | 67 |
| Congested Time (number of "Rush Hours") | 7.6 | 7.6 | 7.6 | 7.4 | 7.4 | 7.4 |
| Annual Increase Needed to Maintain Constant Congestion Level: |  |  |  |  |  |  |
| Lane-miles | 78 | 57 | 87 | 97 | 106 | 112 |
| Transit Riders or Carpoolers (millions) | 26 | 20 | 30 | 33 | 35 | 37 |
| Annual Excess Fuel Consumed |  |  |  |  |  |  |
| Total Fuel (1000 gallons) | 27,842 | 27,455 | 26,342 | 25,754 | 25,657 | 26,094 |
| Rank | 23 | 23 | 23 | 23 | 22 | 21 |
| Fuel per Peak Traveler (gallons) | 35 | 36 | 35 | 36 | 36 | 38 |
| Rank | 9 | 9 | 10 | 7 | 7 | 6 |
| Annual Delay |  |  |  |  |  |  |
| Total Delay (1000s of person-hours) | 41,791 | 41,682 | 40,990 | 40,463 | 40,835 | 41,038 |
| Rank | 22 | 22 | 22 | 22 | 21 | 21 |
| Delay per Peak Traveler (person-hours) | 53 | 55 | 55 | 56 | 58 | 61 |
| Rank | 6 | 6 | 6 | 4 | 4 | 2 |
| Delay due to Incidents (percent) | 53 | 53 | 53 | 53 | 53 | 53 |
| Travel Time Index | 1.30 | 1.31 | 1.30 | 1.30 | 1.31 | 1.32 |
| Rank | 17 | 14 | 19 | 14 | 10 | 9 |
| Congestion Cost |  |  |  |  |  |  |
| Total Cost (\$ millions) | 850 | 818 | 766 | 716 | 688 | 675 |
| Rank | 22 | 22 | 22 | 22 | 21 | 21 |
| Cost per Peak Traveler (\$) | 1,080 | 1,070 | 1,020 | 988 | 976 | 995 |
| Rank | 8 | 8 | 7 | 5 | 4 | 3 |

Note: System Performance statistics for 2000 through 2007 data reflect the effects of operational treatments.
Note: Zeroes in the table reflect values less than 0.5.

## Appendix C

Signal Retiming Project Costs

| Roadway | From | To | Project Cost |
| :---: | :---: | :---: | :---: |
| Alafaya TI (SR 434) | Challenger Pkwy Intersection |  | *included with SR 50 |
| Aloma Ave (SR 426) | Phelps Ave | Palmetto Ave | \$33,800 |
| Colonial Dr E. (SR 50) | Murdock Blvd | Avalon Park Blvd | \$49,300 |
| Conway Rd (SR 15) | Michigan Ave | Hoffner Ave | \$27,300 |
| Goldenrod Rd (SR 551) | Bates Rd | Charlin Pkwy | \$41,700 |
| Orange Ave (SR 527) | Drennen Rd | Nela Ave | \$42,800 |
| Semoran Blvd (SR 436) | Aloma Ave | Baldwin Park St | \$21,900 |
| Curry Ford Rd (SR 552) | Conway Rd | Woodgate Blvd | \$27,300 |
| Hoffner Ave/Narcoossee Rd (SR 15) | Goldenrod Rd | Lee Vista Blvd | Next Year |
| Semoran Blvd (SR 436) | Dahlia Dr | T.G. Lee Blvd | \$76,200 |
| Colonial Dr (SR 50) | Mills Ave | Old Cheney Hwy | \$57,600 |
| Mills Ave (SR 15/600) | Marks St | Lake Shore Dr/Rollins St | Next Year |
| Central Blvd | Brown Ave | Mills Ave/Thorton Ave | Next Year |
| US 17/92- US 441 - OBT | Osceola Pkwy | Columbia Ave | \$16,900 |
| US 192 - US 441 | Denn John Ln | Turnpike NB Ramp | \$30,600 |
| US 17/92 | SR 46 (1st St) | 3rd St | *\$20,500 |
| SR 434 | Sand Lake Rd | Jamestown Blvd | *\$30,400 |
| SR 434 | Tollgate Trail | Wayman St | \$31,300 |
| SR 434 | Mitchell Hammock Rd | Palm Valley Dr | \$25,400 |
| SR 436 | Wilshire Dr | Casselton Dr | \$45,950 |
| SR 46 | Park Dr | Sanford Ave | *included with US 17/92 |
| CR 46A | SR 417 NB Ramp | SR 417 SB Ramp | *included with SR 434 |
| SR 426 | Old Howell Branch Rd | Dean Rd | \$36,000 |
| Red Bug Lake Rd | SR 417 EB | SR 417 WB | *included with SR 434 |

## Notes:

1. The above project costs were provided by FDOT
2. The project cost (cells highlighted in the same color under the "Project Cost" column) for each project is prorated based on the number of signals on the study segment
